Future of Geohazards

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There will be Greater Demand for Quantitative Risk Estimates

- Owners and regulators are asking for risk estimates
 - Go beyond compliance with minimum requirements in current practice
 - What is the risk that we are taking?
 - Risk scoring is most common for geohazards
 - Objective to move to quantitative risk estimates including epistemic uncertainties
- Currently, not very good at risk for geohazards
 - Conservatism commonly used in geotechnical engineering
 - Inconsistent treatment of variability between PSHA, site response on structural response
 - Inconsistent levels of complexities in the different steps
 - PSHA practice oriented to buildings, not geohazards
- Risk estimates have very large epistemic uncertainties
 - Decision making under large uncertainties needs to be addressed

Issues for Risk for Geohazards

- For risk estimates, the models and inputs should be mean centered with uncertainty
 - Conservatism is common in geotechnical engineering
 - Conservatism does not fit with probabilistic risk approach
 - Requires some change in geotechnical engineering practice

Example: Removing Conservatism from Risk Estimates



Risk of Dam Failure (Uncontrolled Rapid Release of Water)

Water Level	Initial	Revised
Normal	1/300 yr	1/2000 yr
Lower 10 ft restriction	1/600 yr	1/12000 yr

Issues for Risk for Geohazards

- Consistent treatment of variability between hazard, geotechnical, and structural evaluations
 - Risk is driven by the tails of the distributions
 - Variability is key to the results
 - Include all variability once, but only once
 - Each field wants to have the full variability in their part of the problem

Where Does the Variability in Spectral Shape Belong?

- Hazard analysis
 - PSHA includes variability as part of the standard deviation (sigma) of the GMM
- Site Response
 - Site-specific site effects (differences from the GMM) are modeled and combined with the PSHA results
- Time history selection
 - How much peak-to-trough variability should be included in the time histories?

M6.4 to 6.7, R=5-15 km, VS30=350-600 m/s 10 SA (g) 0.1 0.01 0.001 0.1 0.01 10 Period (sec)

Issues for Risk for Geohazards

- Levels of complexity in each part of the problem
 - Each field tends to oversimplifies the other parts of the problem and makes detailed studies for their topic
 - Ground motion / hazard
 - Geotechnical
 - Structural
 - Goal to have a comparable level of complexity in each part of the problem

Levels of Complexity

- Example 1: Putting all the complexity into the model for the dam
 - Very complicated finite-element model of a dam
 - Long run times, so use only one time history
- Example 2: Putting all the complexity into the ground motions
 - 100s of sets of time histories to capture the full hazard
 - Many calculations, so use a very simple model for the dam
- Combined:
 - Use the simplified dam model with 100s of time histories to select a small set of time histories that represent the mean and variability of the response of the dam
 - Use this small set of time histories with the complex dam model

PSHA for Geohazards

- PSHA usually conducted for pseudo-spectral acceleration (PSA)
 - Design ground-motion levels and time histories chosen based on UHS or CMS for PSA
 - Easy to do
 - May work well for buildings
 - May be misleading for geohazards
- Secondary parameters considered for geohazards
 - Duration, I_a, CAV, PGV, ...
 - Deformation for simplified (proxy) systems
- Should compute hazard and select time histories based directly on these other parameters that are more closely related damage to geotechnical systems
 - Stop treating geohazard as if they are buildings

PSHA

- Key concept of PSHA is that we are summing the rates of damaging motions from all earthquake scenarios
- Applicable if we are adding the rates of similar loading
 - Damaging and non-damaging rates should not be summed
 - If PSA(T) is not a good measure of damage for geohazard, then we should not be summing the hazard from different sources
 - In this case, the hazard should be computed for a different IM

IMs for Geohazards Applications

- Most of the GMM developed are for PSA(T)
 - PEER NGA projects are Large multi-year, multiinvestigator efforts
 - Resulting in sets of alternative GMMs that have been well reviewed
 - Physical constraints on the extrapolation outside the data range
- For other IMs, fewer GMM available
 - May not be consistent with scaling in the PSA (T) models

Conditional GMM for Geohazards Applications

- Conditional GMMs use the observed PSA(T) as an input parameter
- Example: I_a model by Macedo et al (2018)
 - $Ln(I_a) = c_1 + c_2 ln(VS30) + c_3 M + c_4 ln(PGA) + c_5 ln(PSA(T=1))$
 - Similar approach used in simplified models for deformation
- Advantages of conditional GMM
 - Physical constraints for extrapolation and more complex scaling (HW, NL site, directivity) are represented in the PSA(T) values
 - Conditional GMM are simple to develop and robust
 - Can be combined with suite of PSA(T) models to get a suite of GMMs for the new IM
 - Can get a set of GMMs for the epistemic uncertainty without waiting for separate traditional models to be developed

Selecting Time Histories Using Contribution to Hazard



Handoff between PSHA and Geohazard

- PSHA uses VS30 as the site class
 - V_{S30} is not the fundamental physical parameter for site amplification.
 - Index of the velocity profile
 - Site class that is continuous
 - For typical strong motion sites in California, V_{S30} correlated with deeper Vs profile that controls the site amplification
 - Most soil sites are in alluvial basins (deep soils)
- Move to providing the VS profile that goes with the GMM and VS30 as the site class
 - Site response should model the differences in the site response for the GMM profile and the site-specific profile

Example of Regional Differences in V_s Profiles for V_{s30} =300 m/s



From Lavrentiadis and Abrahamson (2019)

Example of Uncertainty Risk for Dams in Norther California



Decision Making with Large Uncertainties

- Uncertainties in risk
 - Hazard uncertainty usually dominates risk uncertainty
 - Uncertainties in fragilities are not well developed and may be underestimated currently.
- What is done?
 - Most common to use the mean risk
 - Single number that is easy to compare to a risk objective
 - Just using mean leads to misleading accuracy
 - Main use is for comparing risk between different structures
 - Some use of the value of mean risk to make retrofit decisions

Decision Making with Large Uncertainties

- What should we do?
 - Communicate the size of the uncertainty to decision makers
 - This uncertainty should be considered along with other engingeering judgments about the project
 - If the size of the uncertainty is understood, it makes the case for doing the research to significantly reduce uncertainties
 - Develop research to improve the models and collect site-specific data to reduce the uncertainties

Summary - Move to quantitative risk calculations for geohazards

- Requires consistent handoff of information between hazard analyst and geotechnical
 - Clear definition of site condition used for hazard (VS profile)
 - Partitioning of the variability between PSHA and site response
- Requires better understanding of how geotechnical systems actually perform
 - What is the expected behavior under a given loading condition?
 - Mean, aleatory variability
 - What is the epistemic uncertainty in the expected behavior?
- Large epistemic uncertainties in risk estimates should be considered in decision making
 - Just using the mean risk is not adequate