



#### Numerical Simulation of Tsunami-Borne Debris Impact Loads on Bridges

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- Understand the two-fold effect of debris: (a) **impact** and (b) **damming** on bridges.
- Decipher (a) debris-wave interaction during tsunami propagation inland and (b) debris-wave-bridge interaction and associated loads, for different debris orientations.
- Explore and calibrate novel **particle-based** (SPH) and/or coupled mesh-particle methods (**SPH-FEM**) and hybrid particle-mesh based (**PFEM**, to be undertaken by OSU)
- Decipher the role of **non-structural** mass.
- Develop **prescriptive load equations** for debris impact forces for inclusion in the *Tsunami Design Guidelines for Coastal Bridges* developed by PEER and recently adopted by the AASHTO Committee on Bridges and Structures.

# Why debris?



Debris-related issues (from left to right): (a) Shipping containers after the 2010 Tsunami in Talcahuano, Chile (Garcia, 2010), (b) Remaining boat debris after



the 2011 Japan tsunami (courtesy: Dr. Unjoh, Public Works Research Institute, Japan)

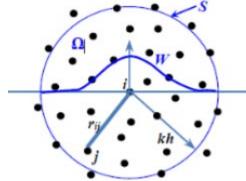
Span removed due to shipping container collision during the 2011 Japan tsunami. (Source: Hoshikuma, PWRI, OSU Tsunami modeling workshop, 2014)

# Methodology

#### Particle-Based; Smoothed Particle Hydrodynamics (SPH)

- Originally invented for solving astrophysical problems in open space.
- Applied to general fluid dynamic problems in early 1990s.
- SPH- Numerical Approximation
- Weight function (or smoothing function), *W*, centered on particles and describe continuous or discrete field function, Kernel approximation:  $\langle f(x) \rangle = \int_0^{\kappa h} f(x') W(x - x', h) dx'$

Particle approximation:  $\langle f(x_i) \rangle = \sum_{j=1}^n \frac{m_j}{\rho_j} f(x_i) W(x_i - x_j, h) = \sum_{j=1}^n \frac{m_j}{\rho_j} f(x_j) W_{ij}$ 



SPH approximation in 2D space

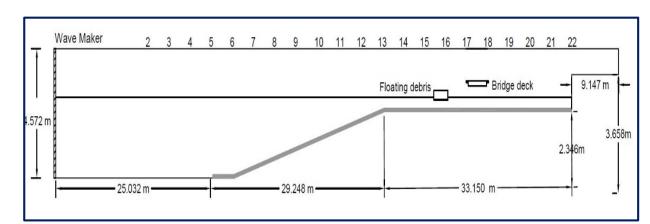
# Methodology (cont.)

> SPH- Equation of Motions for Fluid - Implemented in LS-DYNA

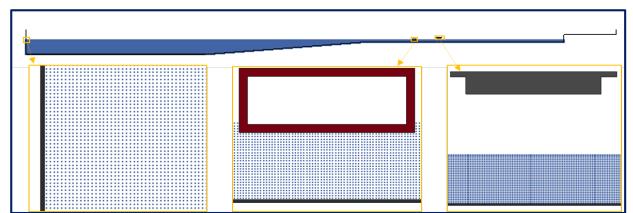
Density 
$$\frac{d\rho_i}{dt} = \sum_{j=1}^n m_j \left(x_i^\beta - x_j^\beta\right) \frac{\partial W_{ij}}{\partial x_i^\beta}$$
Momentum 
$$\frac{dv_i^\alpha}{dt} = \sum_{j=1}^n m_j \left(\frac{\sigma_i^{\alpha\beta}}{\rho_i^2} + \frac{\sigma_j^{\alpha\beta}}{\rho_j^2}\right) \frac{\partial W_{ij}}{\partial x_i^\beta}$$
Energy 
$$\frac{de_i}{dt} = -\frac{p_i + \pi_{ij}}{\rho_i^2} \sum_{j=1}^n m_j \left(v_i - v_j\right) \frac{\partial W_{ij}}{\partial x_i^\beta}$$
Strain 
$$\frac{dx_i^\alpha}{dt} = v_i + \varepsilon \sum_{j=1}^n \frac{m_j}{\rho_j} \left(v_i - v_j\right) W_{ij}$$

### Multi-physics modeling of tsunami debris impact on bridge decks

- Wave maker, flume, debris and bridge: Finite Element, Mesh size=1cm
- Fluid: **SPH**, Particle size= 1cm
- Interaction between SPH and FE parts: Node-to-solid contact

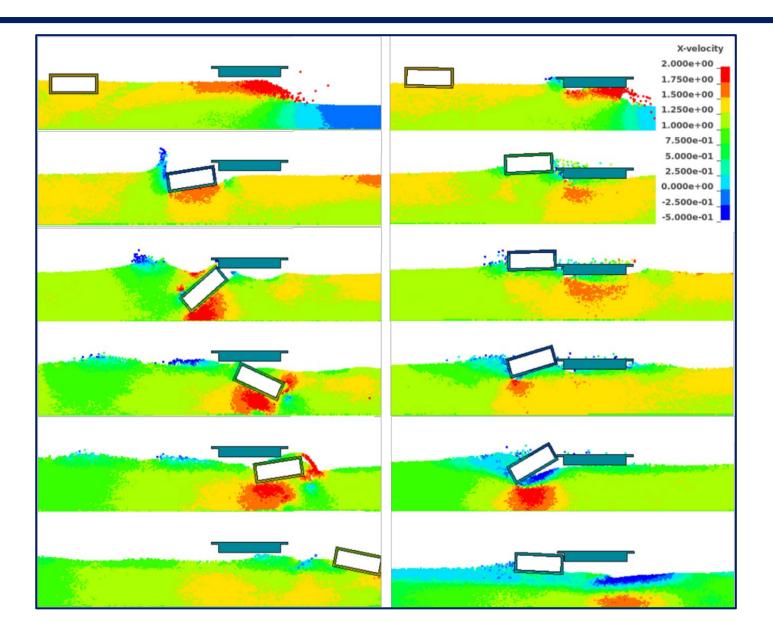


- Interaction between the FE parts: Two-way segmentbased contact
- Final numerical model consisted of 17,077 shell elements and **1,328,633** SPH particles.
- Using up to 80 cores per analysis, the run-time for each model was **19hrs**.



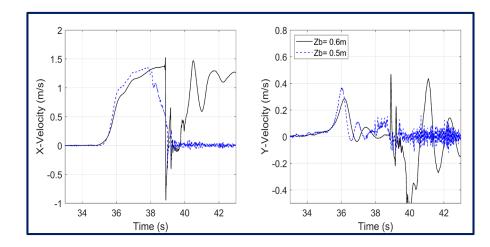
### Multi-physics modeling of tsunami debris impact on bridge decks

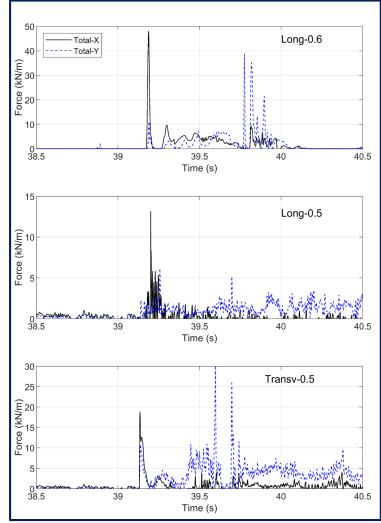
- Debris can either **move below the deck** or **become trapped below the overhang**.
- Complex debris flow pattern is function of the deck elevation and the bore properties.
- Is it possible to predict in advance the debris motion? Such information would be essential for future risk assessment frameworks.



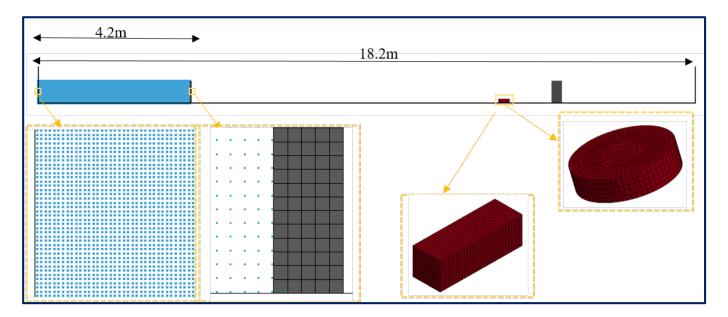
### Multi-physics modeling of tsunami debris impact on bridge decks

- The debris has **vertical impact velocity** with a magnitude in the range of (20-27)% of the horizontal velocity.
- There are several vertical impact loads on the superstructure after the primary impact, with magnitudes up to 4 times larger than the initial impact.
- Horizontal and vertical debris loads are maximized at different instants. Consider multiple load cases to identify the critical case.



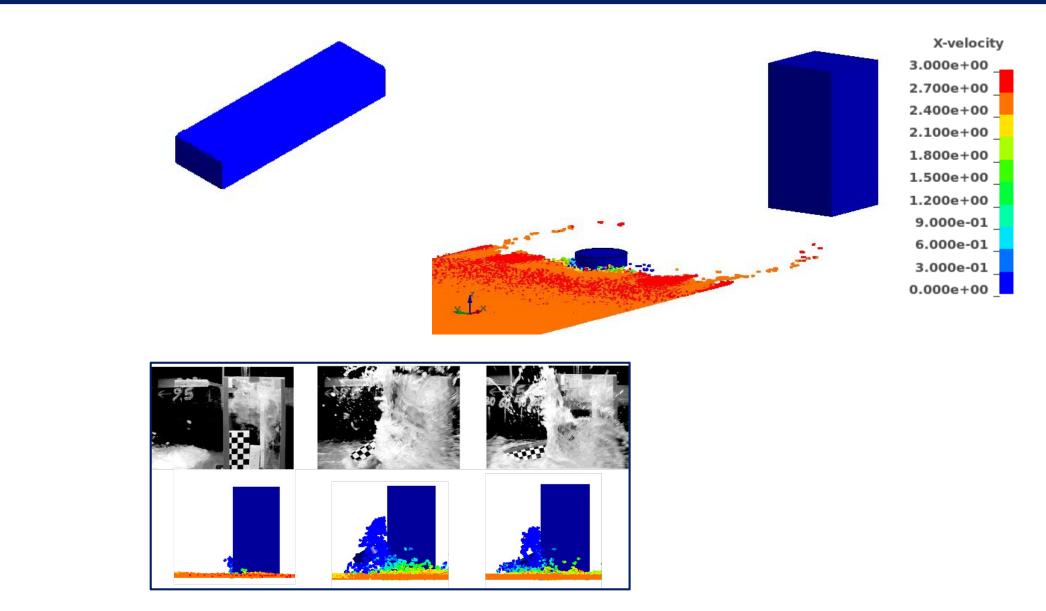


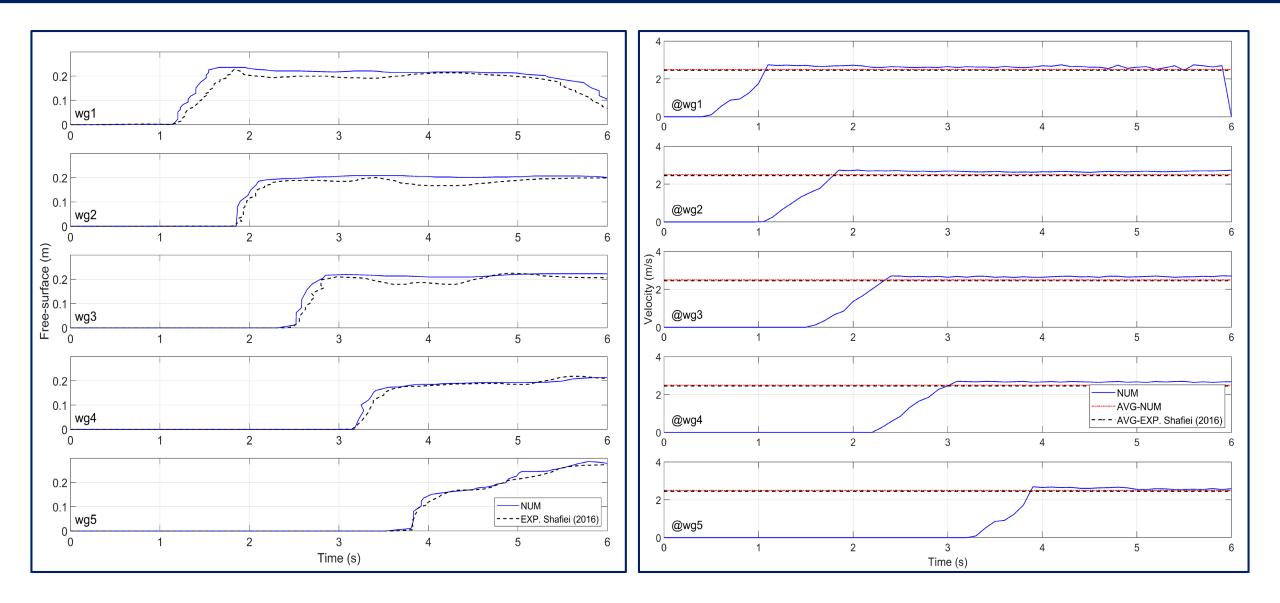
- Source (Shafiei 2016)
- The experiment was conducted in a 14m long, 1.2m wide and 0.8m deep wave flume at the University of Auckland.
- The coastal structure was represented by a square prism.
- Rigid acrylic box and disk with dimensions of 0.1\*0.1\*0.3m and 0.2 (diameter)\* 0.05 (thickness) m were used as the debris.
- To measure the free-surface and fluid velocity, five wave gages were installed along the flume.

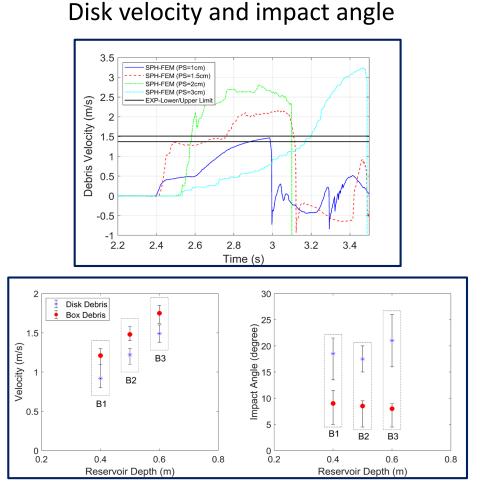


Shafiei (2016). Tsunami Inland Structures Interaction and Impact of Floating Debris (Doctoral dissertation, University of Auckland).

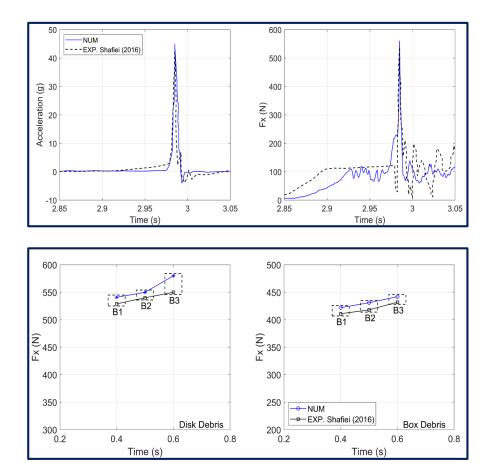
- Gate, flume, debris and structure: Finite Element, Mesh size=1cm
- Fluid: **SPH**, Particle size= 1cm
- Interaction between SPH and FE parts : NODES\_TO\_SURFACE
- Interaction between FE parts : SURFACE \_TO\_SURFACE
- Three different bores named weak (B1) with velocity of 1.98m/s and height of 140mm, moderate (B2) with velocity of 2.2m/s and height of 170mm and strong (B3) with velocity of 2.48m/s and height of 210mm were generated.
- Final numerical model consisted of 632,848 shell elements and **3,049,163** SPH particles.
- Using up to 80 cores per analysis, the run-time for each model ranged between 27 and 43hrs, depending on the hydrodynamic characteristics.







Debris acceleration and force in the pier



•Velocity: Maximum deviation ranged between (1.5-3)% for the disk and (1-1.5) % for the box.

• Impact angle: Maximum deviation ranged between (3-5)% for the disk and (2-6) % for the box.

•Impact force: Maximum deviation ranged between (1.8-5.5)% for the disk and (2.5-3.11) % for the box.

The **validated 3D numerical tank** was used for the parametric investigation, with some additional modifications:

- The coastal structure was replaced by a bridge deck which follows the dimensions of the deck used in previous large-scale hydrodynamic test (Istrati 2017).
- The floating debris was represented by a standard shipping container .
- The length of the reservoir for the numerical investigation was increased from 4.2m to 10m.
- The outlet was moved 2m further upstream of the bridge and an artificial beach with slope of 1:12 with end reservoir were added to the numerical model.

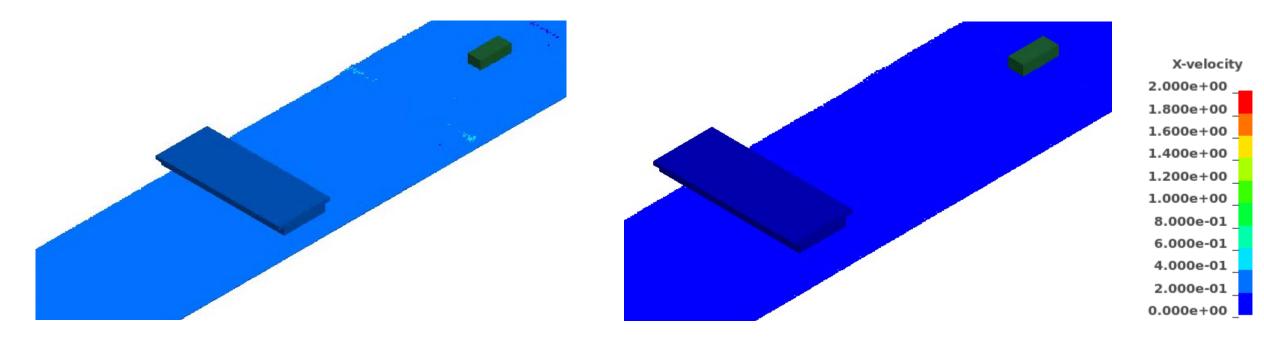
- Gate, flume, debris and structure: Finite Element, Mesh size=1cm
- Fluid: **SPH**, Particle size =1cm
- Scale: 1/20
- Interaction between fluid and solid body: NODES\_TO\_SURFACE
- Interaction between the solid bodies: SURFACE \_TO\_SURFACE
- Final numerical model consisted of 969,908 shell elements and 4,203,331 SPH particles.
- Using up to 80 cores per analysis, the run-time for each model ranged between 78 and 110hrs, depending on the hydrodynamic characteristics.

Investigate the effect of:

- Bridge elevation (0.20, 0.30 and 0.35m)
- Debris orientation (longitudinal and transverse)
- Debris mass
- Hydrodynamic conditions (H<sub>res</sub> = Reservoir depth, d= Still-water depth)

| Hydrodynamic Conditions |          |       |
|-------------------------|----------|-------|
| Bore Cases              | Hres (m) | d (m) |
| <b>B1</b>               | 0.40     | 0.10  |
| B2                      |          | 0.15  |
| B3                      | 0.60     | 0.075 |
| <b>B</b> 4              |          | 0.10  |
| B5                      |          | 0.15  |
| B6                      |          | 0.20  |
| B7                      |          | 0.25  |

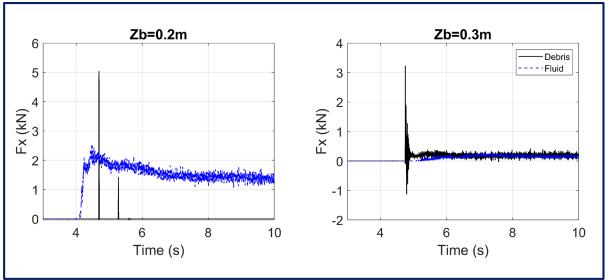
Role of **deck elevation**: B4:  $H_{res} = 0.60m$ , d = 0.10m, deck elevation ( $Z_b$ ) : 0.20 and 0.30m

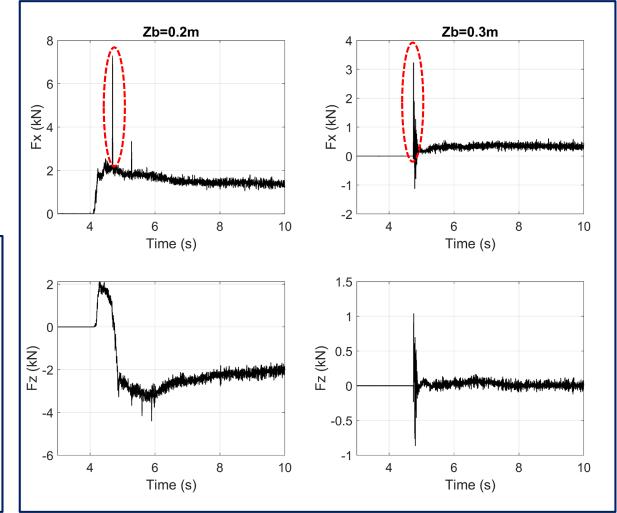


B4- Deck Elevation: 0.20m

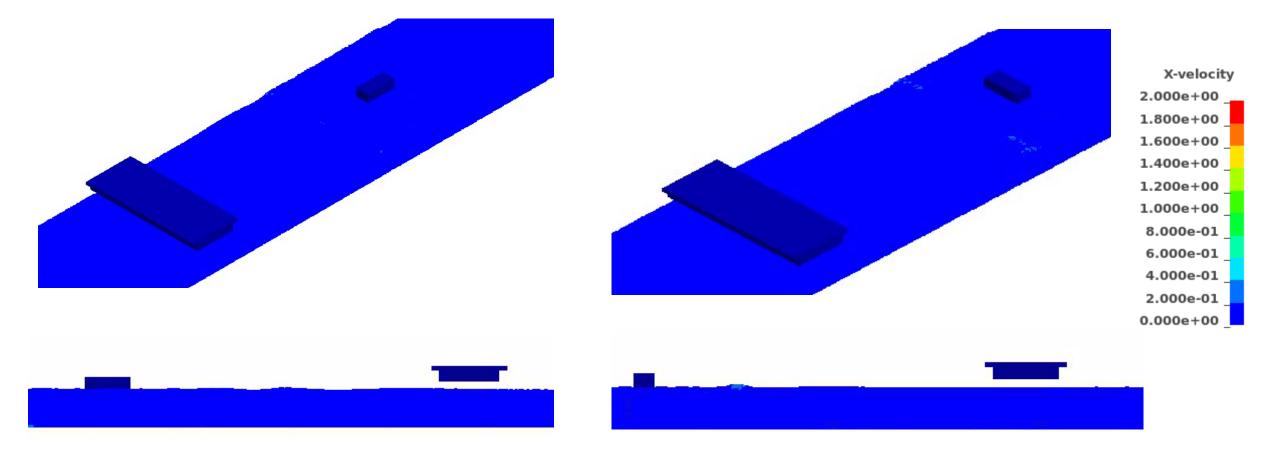
B4- Deck Elevation: 0.30m

- Depending on the deck elevation, ratio of the maximum debris/fluid force ranged between 2.5 up to 10.
- Debris impact vertical load depends on the deck elevation. For higher deck elevations (e.g. Z<sub>b</sub> =0.3m) the uplift load comes only from the debris.





Role of **debris orientation**: B7: H<sub>res</sub> =0.60m, d=0.25m, deck elevation (Z<sub>b</sub>) : 0.30m

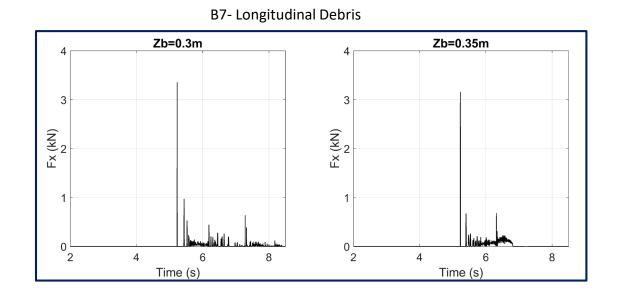


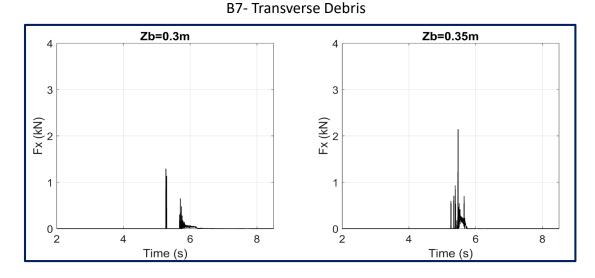
**B7-** Longitudinal Debris

**B7-** Transverse Debris

Role of debris orientation: B7:  $H_{res} = 0.60m$ , d=0.25m, Deck elevation ( $Z_b$ ) : 0.30m

- Preliminary data shows debris orientation has significant effect on (a) the debris motion around the bridge (e.g. if it will move above or below the superstructure), and (b) the peak applied load (Fx).
- The ratio of the **Fx\_longitudinal/Fx\_transverse** ranged between **1.47 to 2.7** (actual ratio is dependent on deck elevation and bore properties).





# Thank you!

