



RC-FIAP

An open platform for
high-resolution structural
design and inelastic modeling for regional
seismic risk assessment

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PEER Annual Meeting
Berkeley, August 24 - 2023



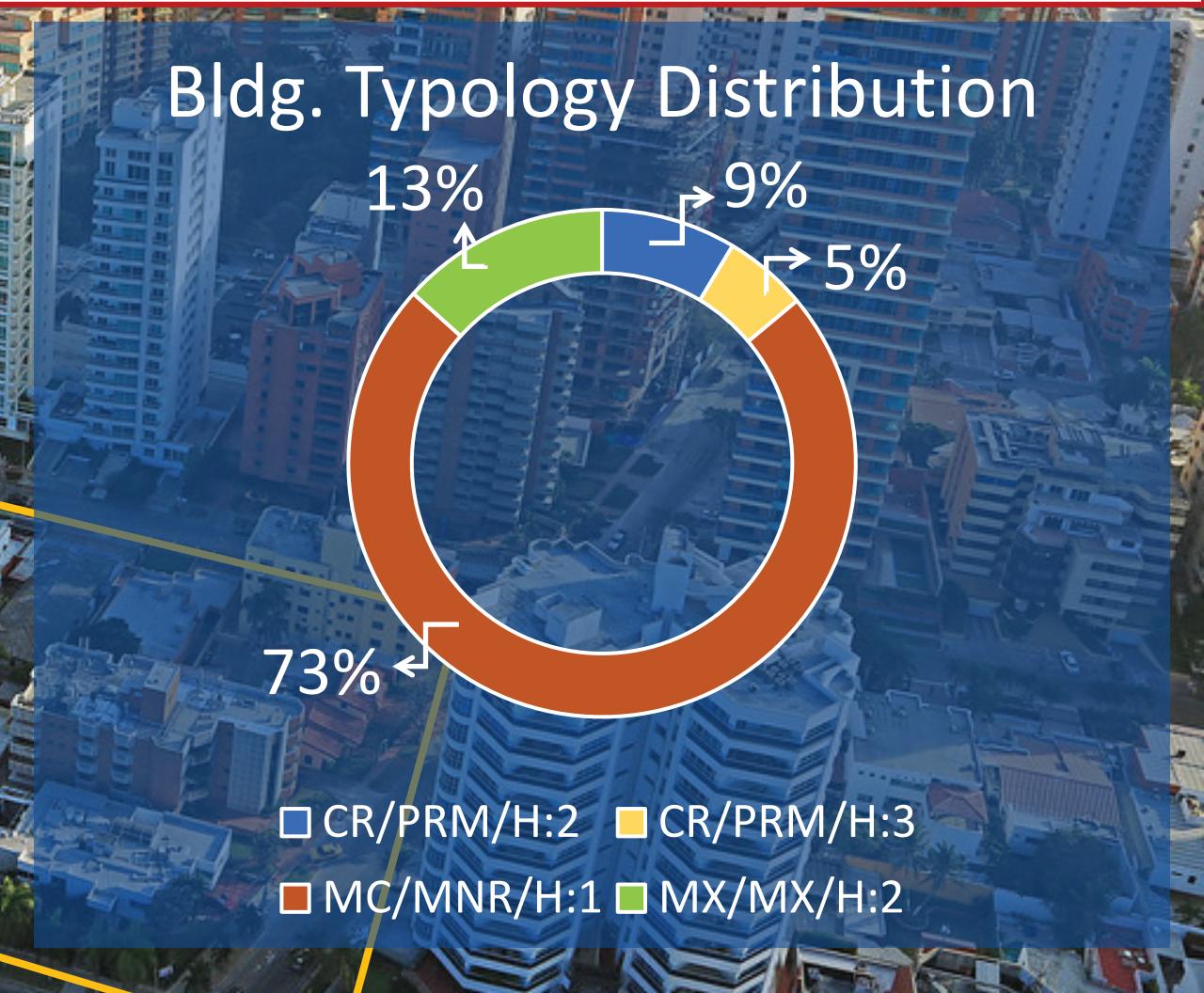
MOTIVATION I: COLOMBIAN NATIONAL RISK MODEL

Task 1: Residential building inventory covering 60% of the population.



MOTIVATION I: COLOMBIAN NATIONAL RISK MODEL

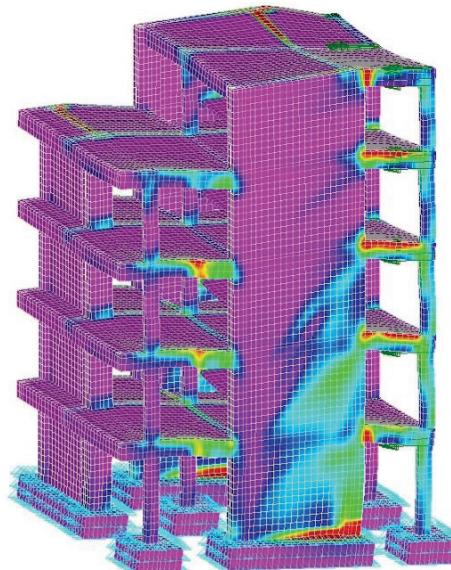
Blockwide resolution



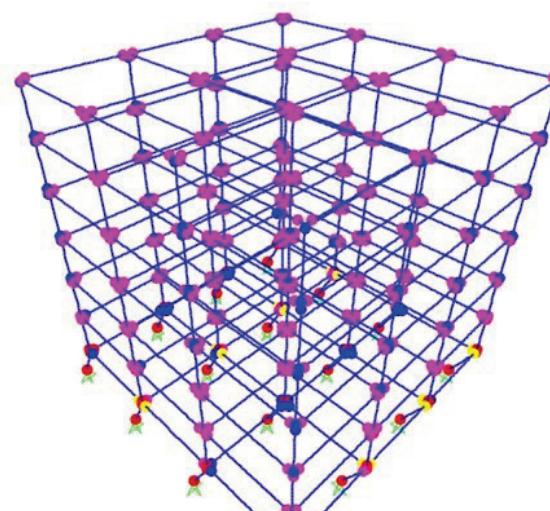
MOTIVATION I: COLOMBIAN NATIONAL RISK MODEL

Task 2: What is **the seismic fragility and vulnerability** of the Colombian RC frame and frame-wall system?

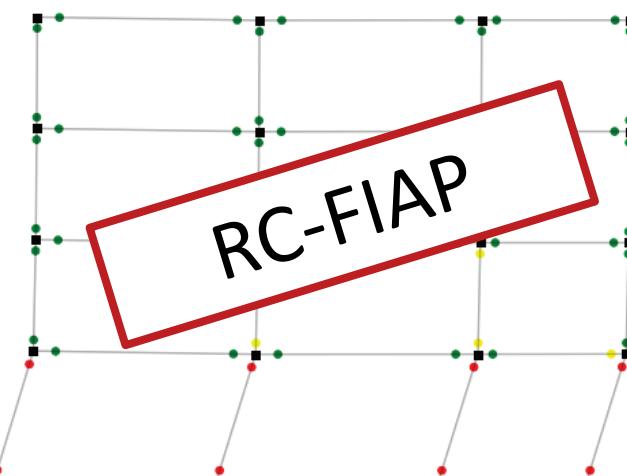
3D NL FEM
MICROMODELS



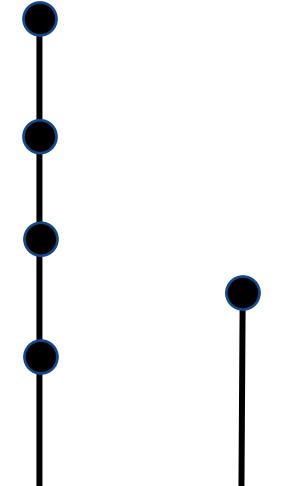
3D NL FEM
MACROMODELS



2D NL MACROMODELS



NL
MDOF NL
SDOF



MOTIVATION II: FACILITATE INTEGRATION OF THE PEER EQ.

1 PEER PBEE Equation

$$\lambda(dv) = \iiint G_{DV} \langle dv | dm \rangle | dG_{DM} \langle dm | edp \rangle | | dG_{EDP} \langle edp | im \rangle | d\lambda(im)$$

2 Fragility Formulation

$$\left\{ \begin{array}{l} P(DS > ds | IM) = \int P(DS > ds | EDP) f(EDP | IM) \\ P(DS > ds | im_i) = \sum_{j=1}^m [P(DS > ds | edp_j) * p(edp_j | im_i)] \end{array} \right. \quad \begin{array}{l} \text{Continuous Formulation} \\ \text{Discrete Formulation} \end{array}$$

3 Loss Estimation by Story

$$\begin{aligned} E[L_T | IM] = & E[L_T | NC, ND, IM] * P(ND) * P(NC | IM) + \\ & E[L_T | NC, D, IM] * P(D) * P(NC | IM) + \\ & E[L_T | C] * P(C | IM) \end{aligned}$$

Expected losses due to:

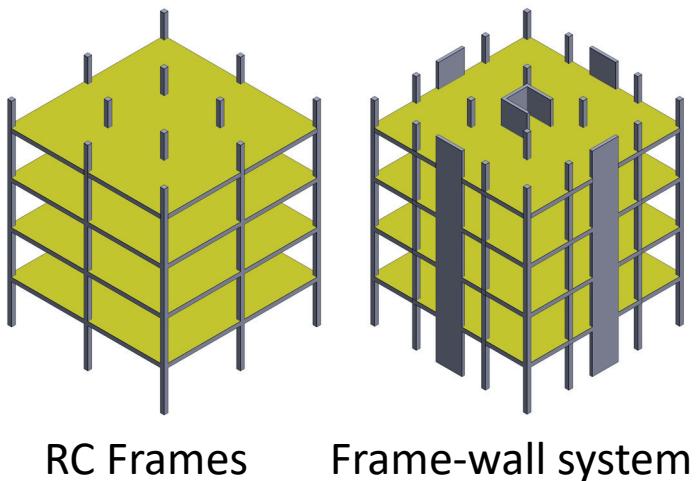
Non-collapse and Non-demolition

Non-collapse and Demolition

Collapse

HOW TO DEVELOP A REGIONAL SEISMIC RISK ASSESSMENT?

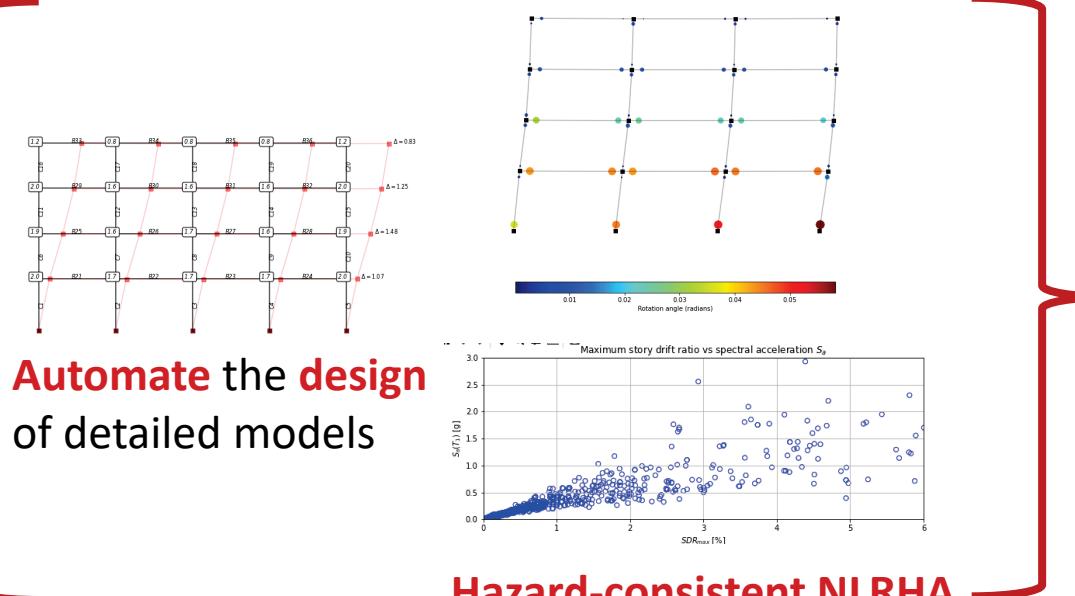
Systems of interest:



RC Frames

Frame-wall system

Automate the design
of detailed models



RC-FIAP
JUST WITH A
CLICK

Fragility of a Building

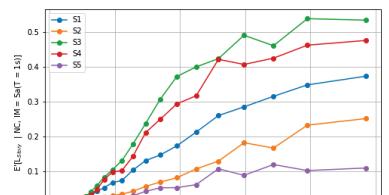


Fragility of a Taxonomy

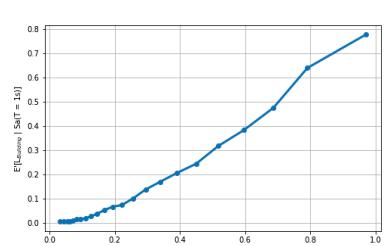


Losses

Story
level

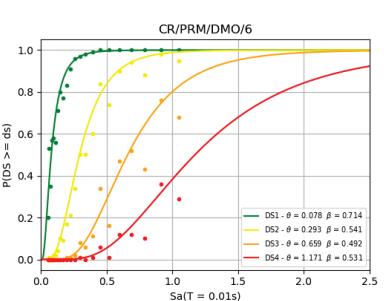
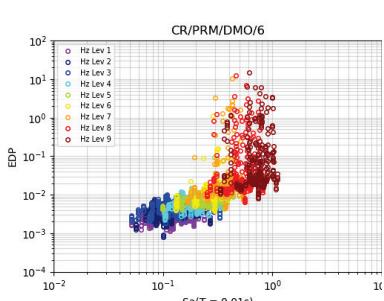
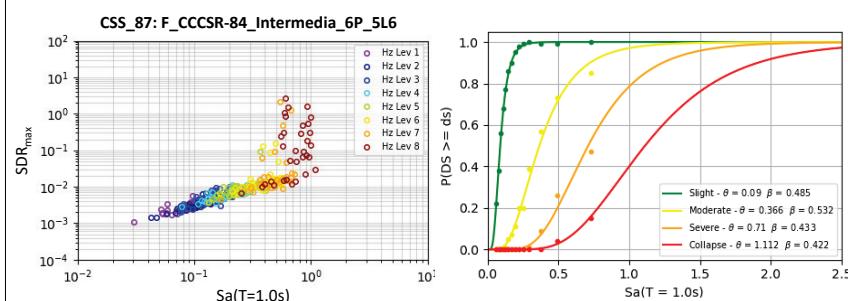


Building
level



V-FAST

Taxonomy: Group of buildings with the same
characteristics



Frame Data Design Results Nonlinear Parameters Nonlinear Analysis Pushover Results IDA Results CSS Results CSS Graphs

Columns sections

Exterior Interior
Width (m) = **0.4** Width (m) = **0.4**
Depth (m) = **0.45** Depth (m) = **0.45**

Beams sections

Width (m) = **0.4**
Depth (m) = **0.4**

Seismic load code and performance factors

R = **5.0** Cd = **4.5** Ω = **3.0** Importance factor = **1.0**

ASCE 7-16

ASCE 7-16 Seismic load parameters

S_{Ds}(g) = **0.357** S_{D1}(g) = **0.185**
T_L(sec) = **4.0**

NSR-10 Seismic load parameters

A_a = **0.15** A_v = **0.20**
F_a = **1.5** F_v = **2.0**

Frame geometryFrame type

Spatial Perimetral

Vector of story heights (m) = h₁,h₂,h₃,..., **3.5,3,3,3,3,3**

Vector of spans (m) = s₁,s₂,s₃,... **6,6,6,6**

Frame tributary gravity loading

Dead load (kN/m²) = **6.5**

6.5

Live load (kN/m²) = **1.8**

1.8

Tributary length for gravity (m) = **6**

6

Tributary length for seismic (m) = **12**

12

PDelta leaning column : Yes No

Inertia ratio I_{LC}/I_{IC} = **1**

| Member | Moment of inertia for elastic analysis |
|---------|--|
| Columns | 0.70 I _g |
| Beams | 0.35 I _g |
| Walls | 0.50 I _g |

Materials

f_y long. (MPa) = **420**

Minimum column-to-beam moment strength ratio = **1.2**

f_y transv. (MPa) = **420**

f_c beams (MPa) = **28**

f_c columns (MPa) = **28**

Design

Creates a linear model in OpenSees and designs it with 1 click!

**INPUT
PARAMETERS
FOR ELASTIC
DESIGN**

C2B Strength Ratios & Design Drifts

Accept design parameters

Accept Design

Beams detailing

| ID | b [cm] | h [cm] | L-end p_top | L-end p_bot | L-end Leg # | L-end Stirrup [cm] | R-end p_top | R-end p_bot | R-end Leg # | R-end Stirrup [cm] |
|-------|--------|--------|-------------|-------------|-------------|--------------------|-------------|-------------|-------------|--------------------|
| 1 B31 | 40 | 40 | 1.254 | 0.368 | 2 | 8 | 1.254 | 0.368 | 2 | 8 |
| 2 B32 | 40 | 40 | 1.254 | 0.368 | 2 | 8 | 1.254 | 0.368 | 2 | 8 |
| 3 B33 | 40 | 40 | | | | | | | 3 | 8 |
| 4 B34 | 40 | 40 | 1.254 | 0.368 | 2 | 8 | 1.254 | 0.368 | 2 | 8 |
| 5 B35 | 40 | 40 | 1.254 | 0.460 | 2 | 8 | 1.463 | 0.460 | 2 | 8 |

Columns detailing

| ID | b [cm] | h [cm] | ρ | db [mm] | dst [mm] | nbH | nbB | Leg # H | Leg # B | SStirrup [cm] | V _u /V _n |
|------|--------|--------|--------|---------|----------|-----|-----|---------|---------|---------------|--------------------------------|
| 1 C1 | 40 | 45 | 1.13 | 12.70 | 9.53 | 5 | 5 | 3 | 3 | 10 | 0.26 |
| 2 C2 | 40 | 45 | 1.13 | 12.70 | 9.53 | 5 | 5 | 3 | 3 | 10 | 0.33 |
| 3 C3 | 40 | 45 | | | | | | | 3 | 10 | 0.34 |
| 4 C4 | 40 | 45 | 1.13 | 12.70 | 9.53 | 5 | 5 | 3 | 3 | 10 | 0.32 |
| 5 C5 | 40 | 45 | 1.13 | 12.70 | 9.53 | 5 | 5 | 3 | 3 | 10 | 0.24 |

Walls detailing

| ID | tw [cm] | lw [cm] | p _l | p _t | db [mm] | dst [mm] | c/lw | σ_c/f'_c | Boundary Elements |
|-------|---------|---------|----------------|----------------|---------|----------|-------|-----------------|-------------------|
| 1 W55 | 30 | 300 | 0.25 | 0.25 | 9.53 | 9.53 | 0.212 | 0.432 | Yes |
| 2 W56 | 30 | 300 | 0.25 | 0.25 | 6.35 | 9.53 | 0.181 | 0.297 | Yes |
| 3 W57 | 30 | 300 | 0.25 | 0.25 | 6.35 | 9.53 | 0.201 | | Yes |
| 4 W58 | 30 | 300 | 0.25 | 0.25 | 6.35 | 9.53 | 0.124 | 0.119 | NO |
| 5 W59 | 30 | 300 | 0.25 | 0.25 | 6.35 | 9.53 | 0.096 | 0.055 | NO |

Frame Data Design Results Nonlinear Parameters Nonlinear Analysis Pushover Results

Plastic hinge models

Columns and beams plastic hinge length l_p Walls plastic hinge length l_p

- $l_p = 0.5H$
- $l_p = 0.08l + 0.022d_b f_y$ (Priestley and Park)
- $l_p = 0.05l + 0.1d_b f_y / \sqrt{f_c}$ (Berry)

- $l_p = 0.2l_w + 0.044(M/V)$ (Priestley)
- $l_p = 0.2l_w + 0.05(M/V)(1-1.5P/(f'_c A_g)) \leq 0.8l_w$ (Bohl)
- $l_p = 0.27l_w(1-P/(f'_c A_g))(1-f_y p_{sh}/f'_c)(M/V/l_w)^{0.45}$ (Kazaz)
- $l_p = \text{average of the above}$

Material regularization

- Concrete
- Steel

Shear model wall

- Linear
- Nonlinear

Steel model

- Hysteretic
- SteelMPF

Element model wall

- ForceBeamColumn
- MVLEM

Joint model

-
-
-
- Linear
- Nonlinear

Shear model frame

- None
- Linear
- Nonlinear

NL Shear model for walls and columns

Infills strength model

NL PARAMETERS GUI

Joint flexibility model

CSS Graphs

Rayleigh damping model

- Based on T1 and T3
- ASCE 41-17 (7.4.4.4)

Target damping ratio (%) = 2.0

Stiffness matrix

- Current
- Initial
- Committed

Damping model for NL RHA

Creates a nonlinear model in OpenSees with 1 click!

Create Nonlinear Model

Pushover analysis options

Frame Data Design Results Nonlinear Parameters Nonlinear Analysis Pushover Results IDA Results CSS Results CSS Graphs

Load pattern
 Triangular Uniform

Type of analysis
 Fast Forced

Drift to plot
 SDR RDR

Alpha curves = **0.75,1.5,3**

Target story drift ratio = **0.02**
Number of steps = **110**
Output Pushover file = **Pushfile_01**

Type of chart
 PHP plastic rotation Acceptance Criteria ASCE-17-41

Run Pushover

Ground motions directory **CSS_BAQ_Soil_T1s**
Output CSS file **CSS_01**
Intensity measure (IM) **Sa(T1)**
Save history results

Run CSS

First intensity (g) **0.05**
Hunting increment step (g) **0.10**
Drift capacity (%) **6**
Maximum number of runs **20**
Seismic records list file **nmsfileX1**
Output IDA file **IDAfile_01**
Intensity measure (IM) **Sa(T1)**

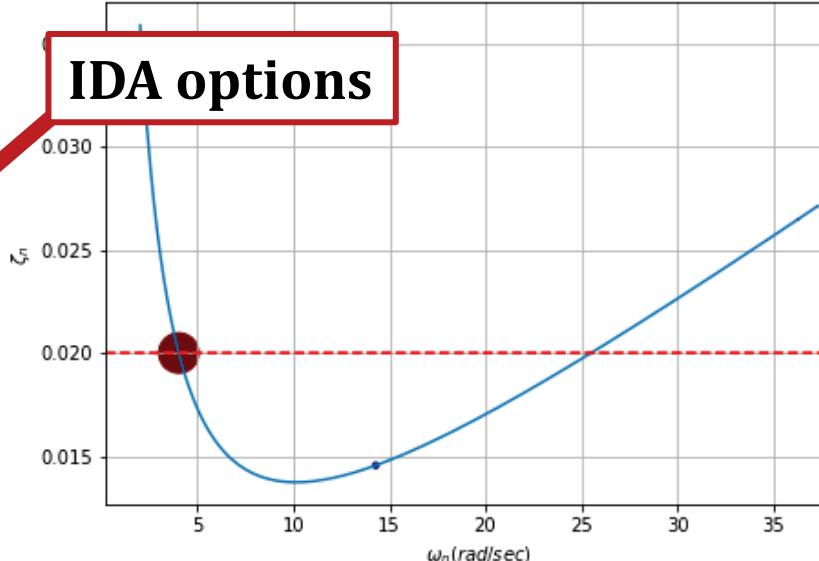
Run IDA

Periods after gravity loads
 $T_1 = 1.56 \text{ sec}$ $T_2 = 0.44 \text{ sec}$ $T_3 = 0.25 \text{ sec}$

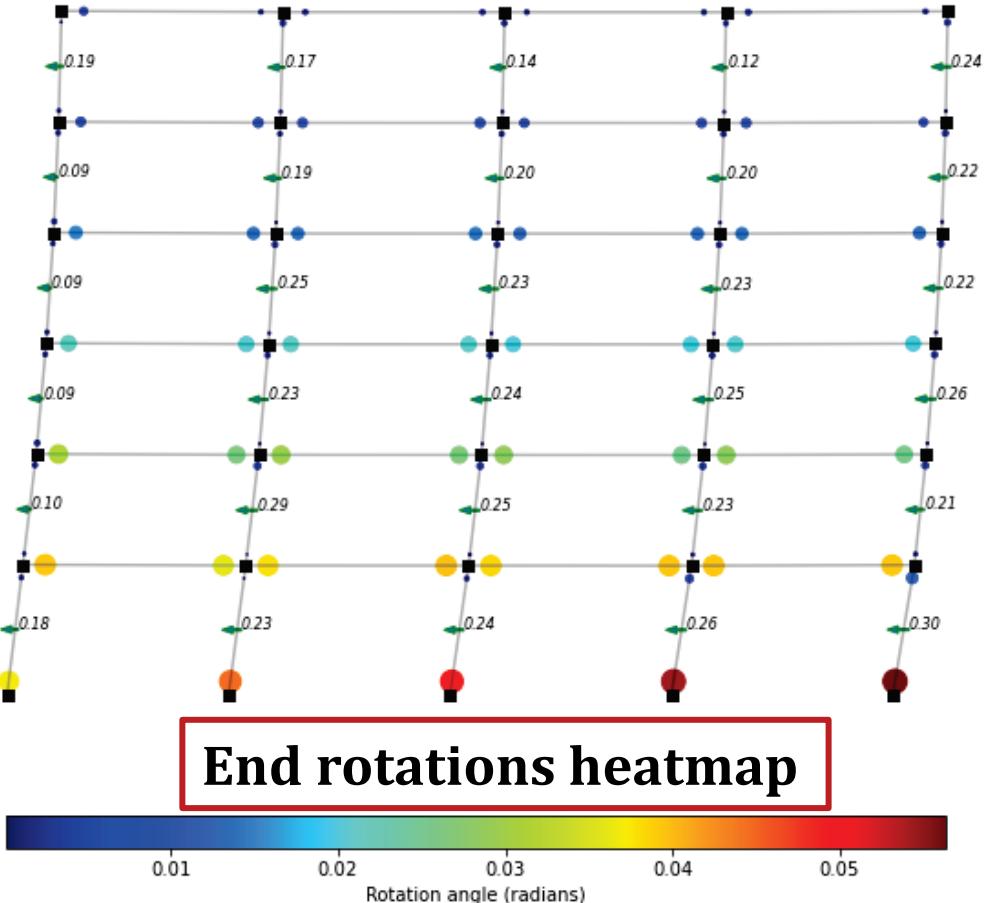
Risk-based analysis options

IDA options

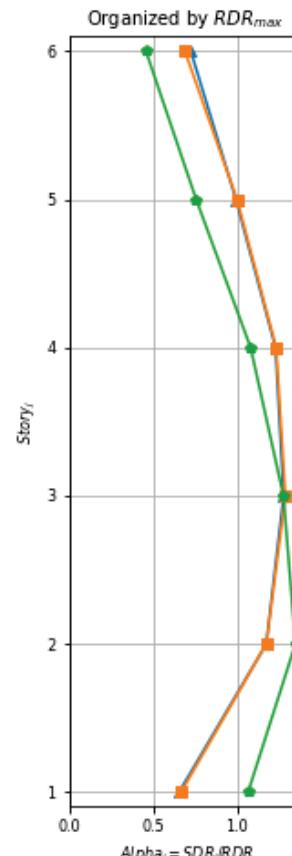
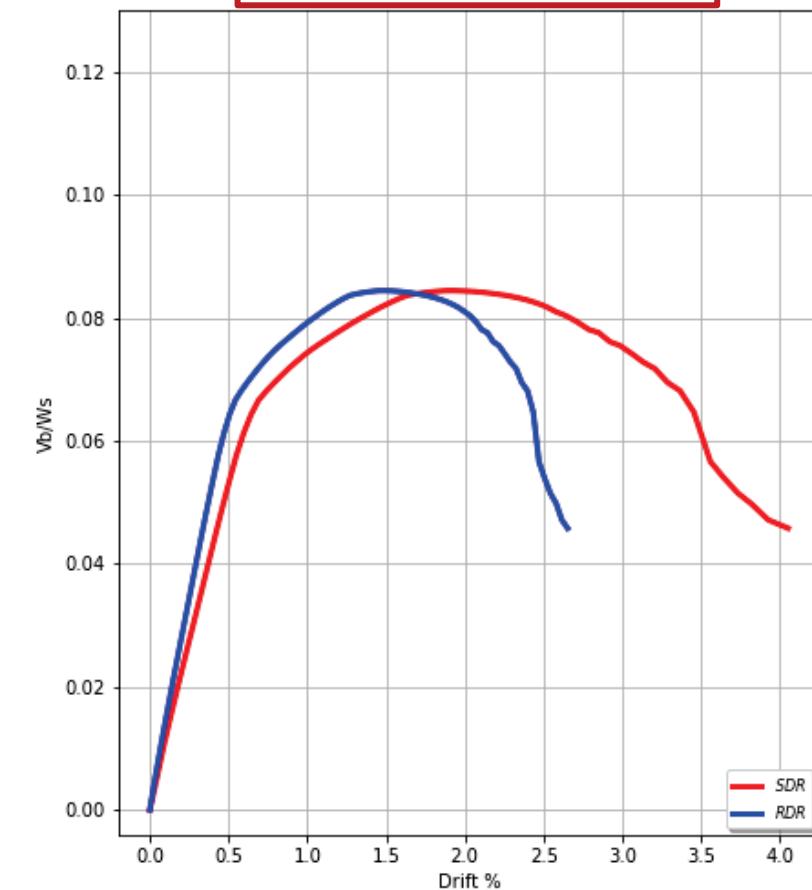
Damping Model: ω_1, ω_3



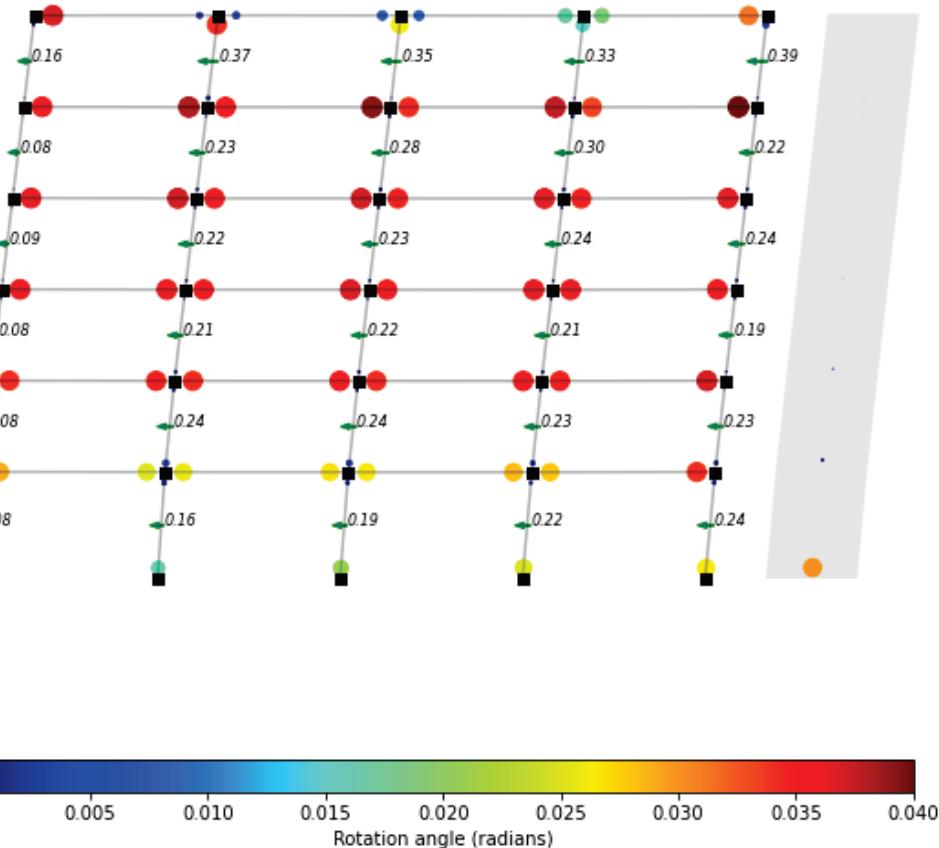
NL ANALYSIS GUI

Plastic hinge projector

$$\alpha = SDR_j / RDR$$

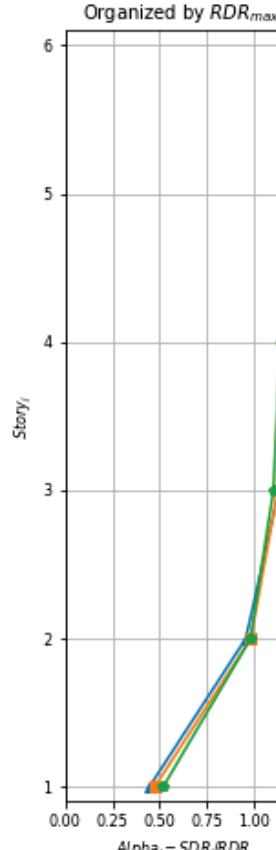
**Pushover curve**

Plastic hinge projector



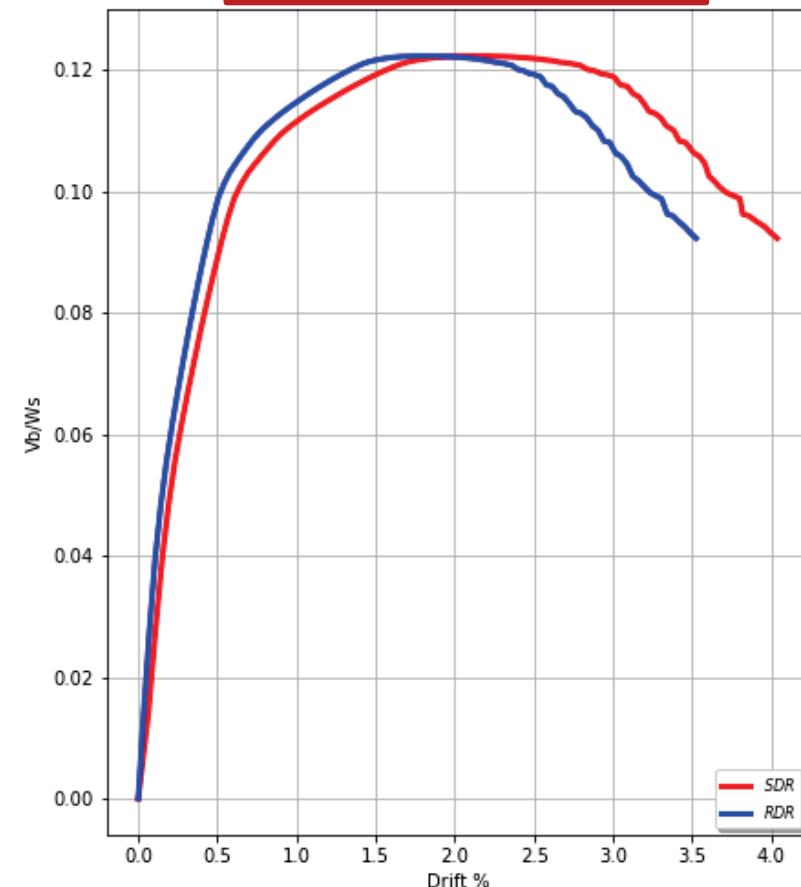
$$\alpha = SDR_j / RDR$$

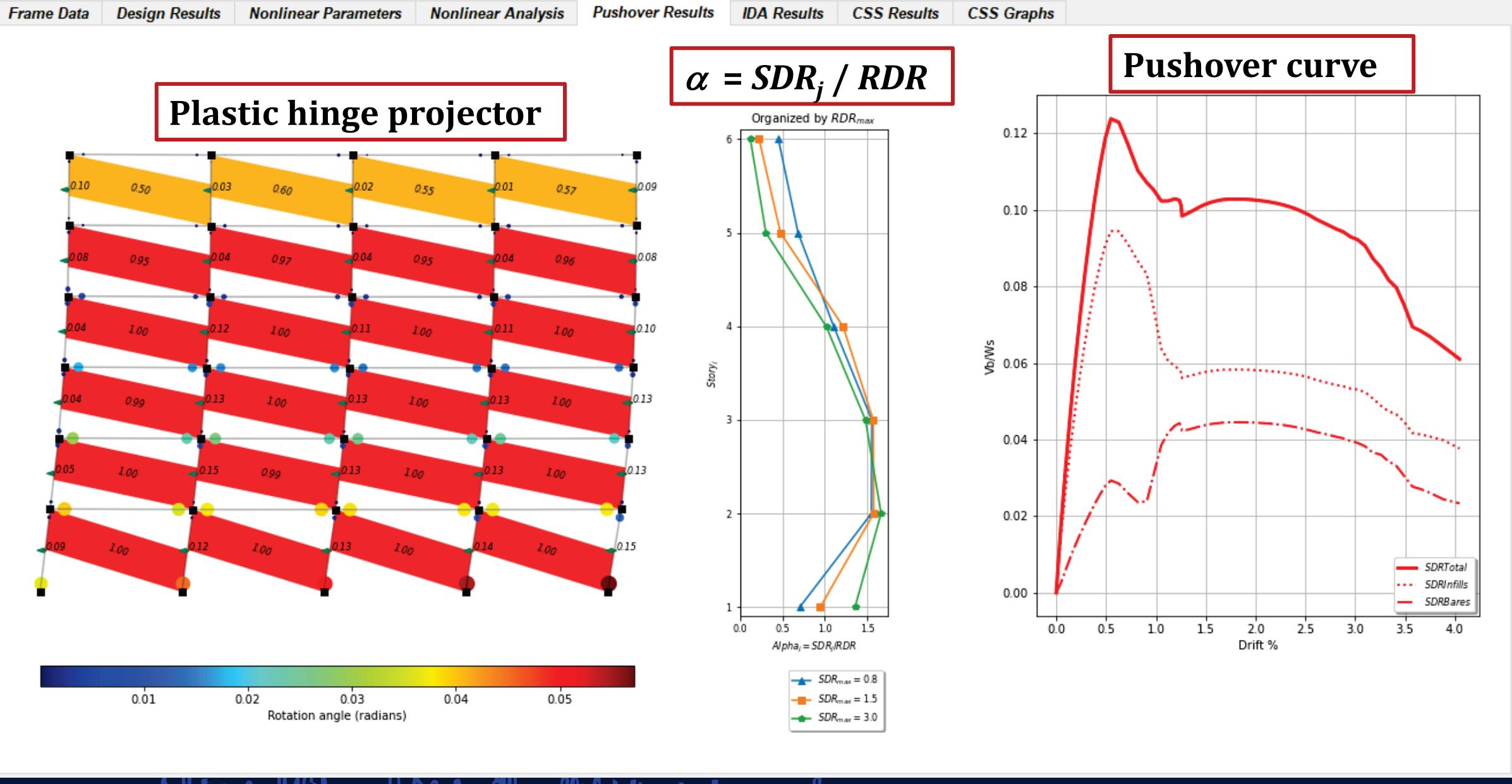
Organized by RDR_{max}



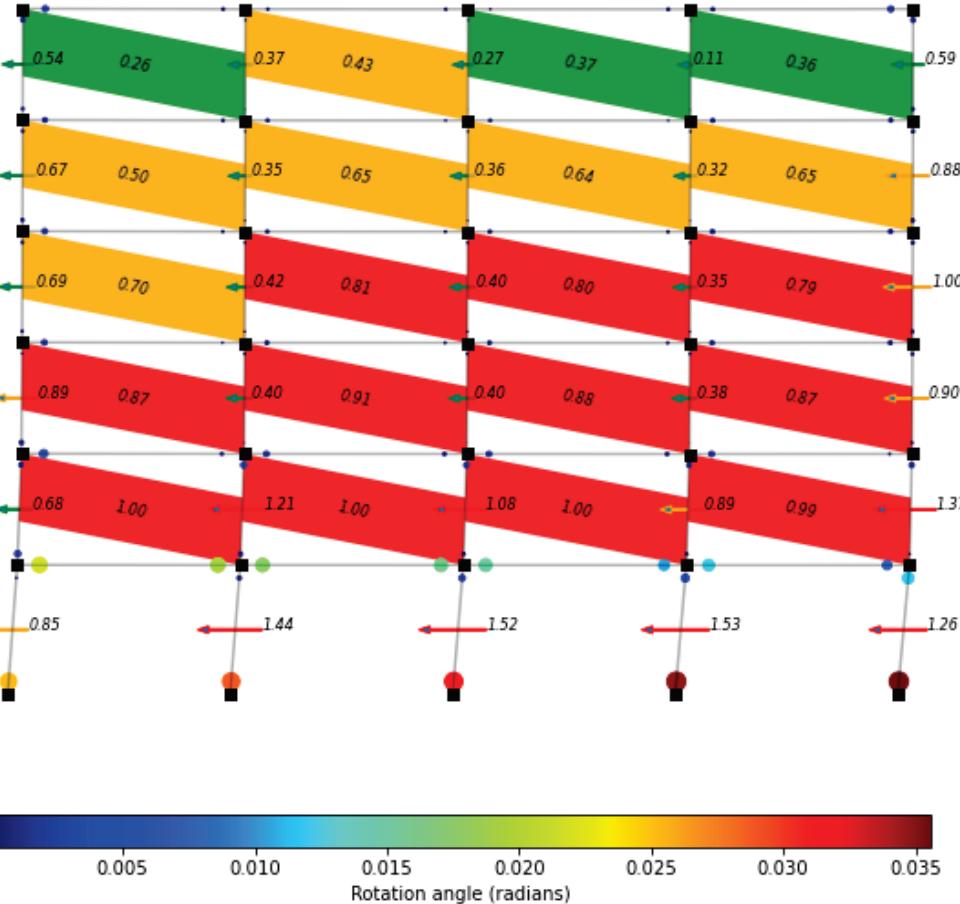
- SDR_{max} = 0.8
- SDR_{max} = 1.5
- SDR_{max} = 3.0

Pushover curve

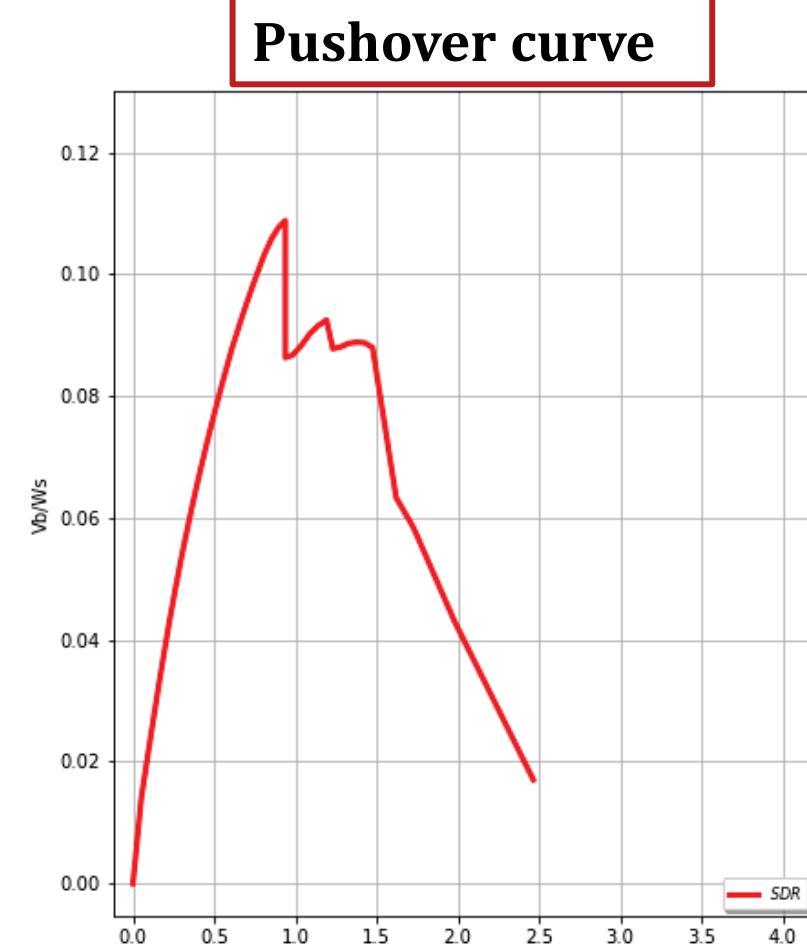


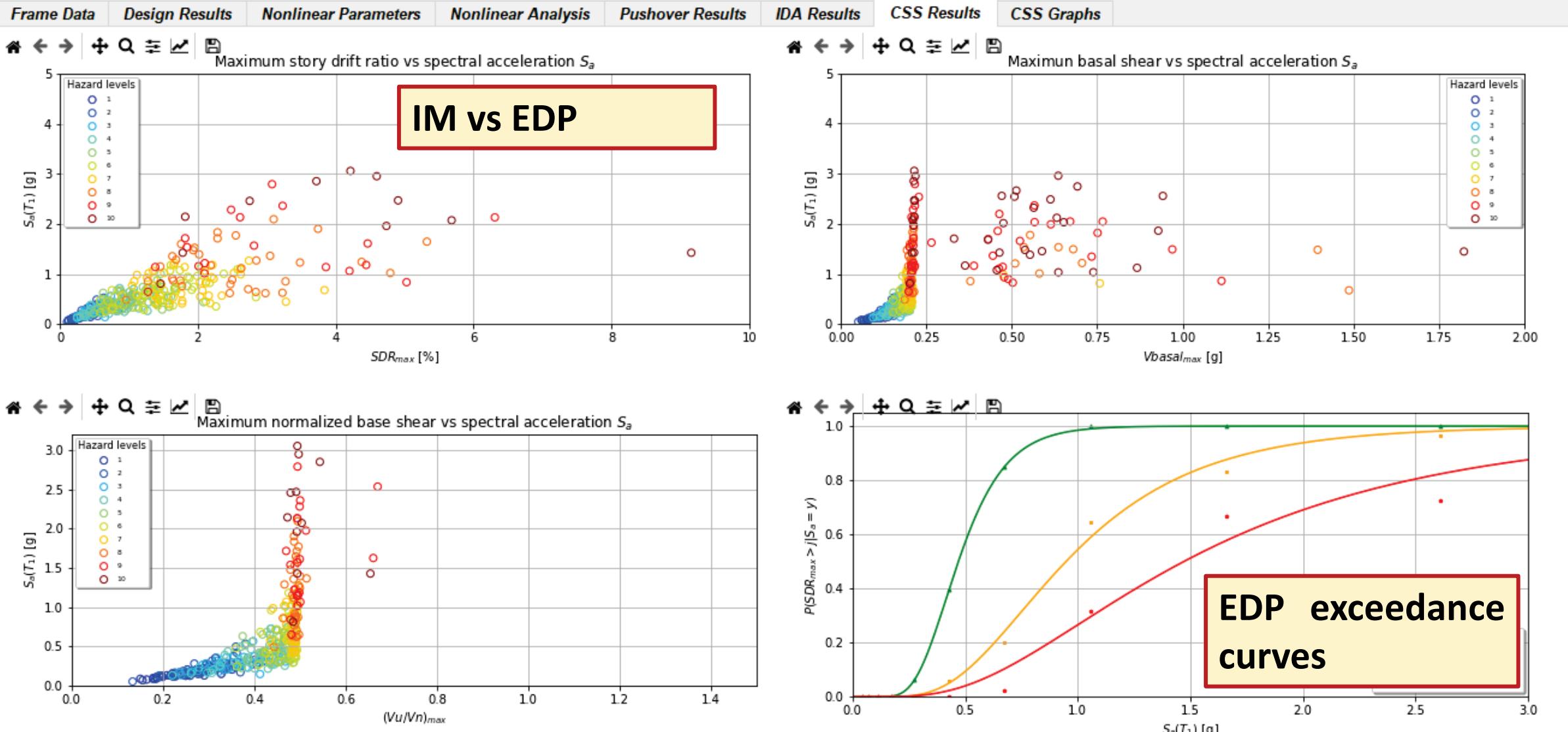


Plastic hinge projector



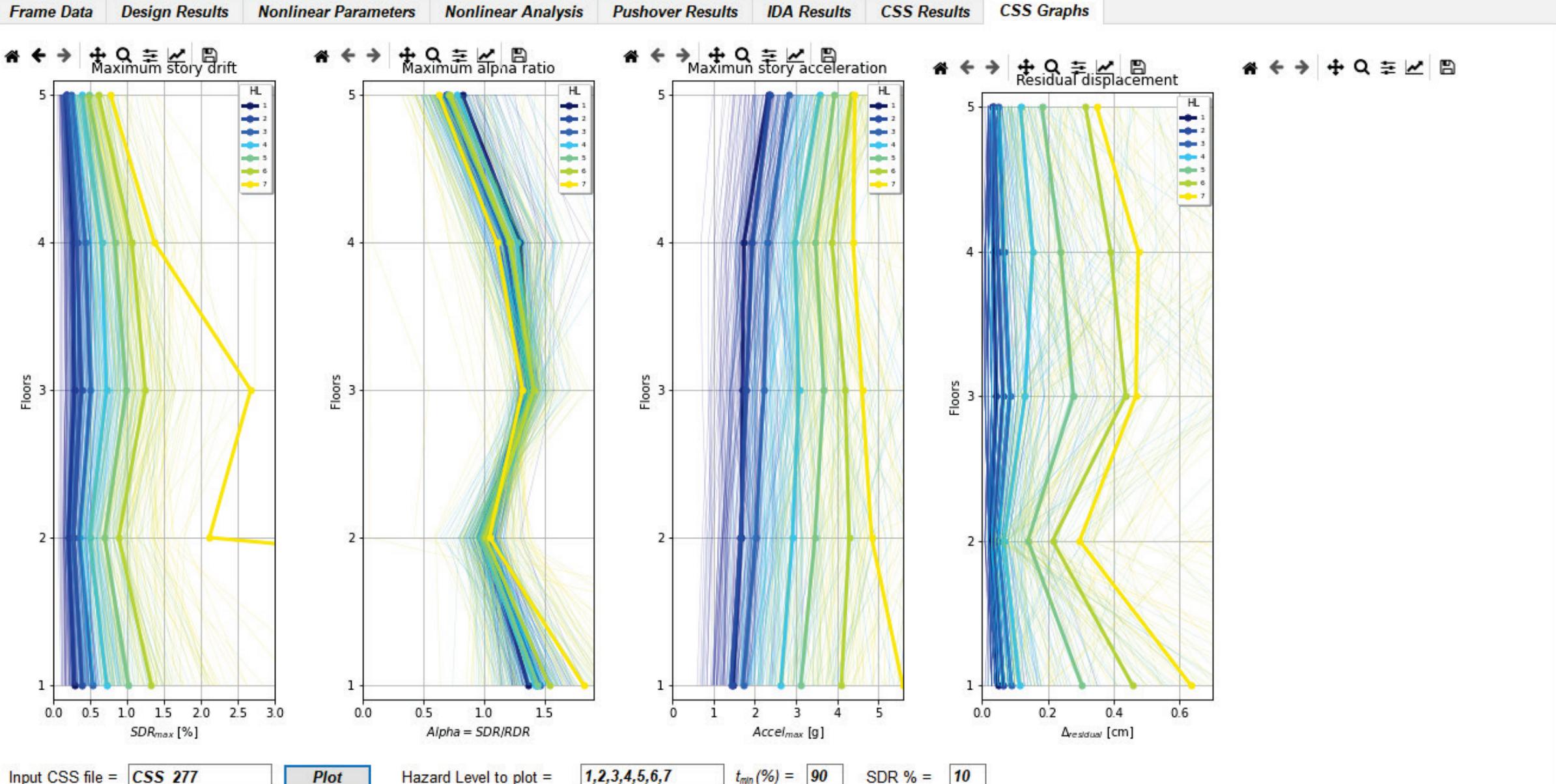
$$\alpha = SDR_j / RDR$$



Input CSS file = **CSS_277**Choose Fragility Curve: **Max SDR**Limit stage % = **1,2,4****Plot**

Hazard Level to plot =

1,2,3,4,5,6,7,8,9,10 **$t_{min} (%)$ = 90**SDR % = **4**

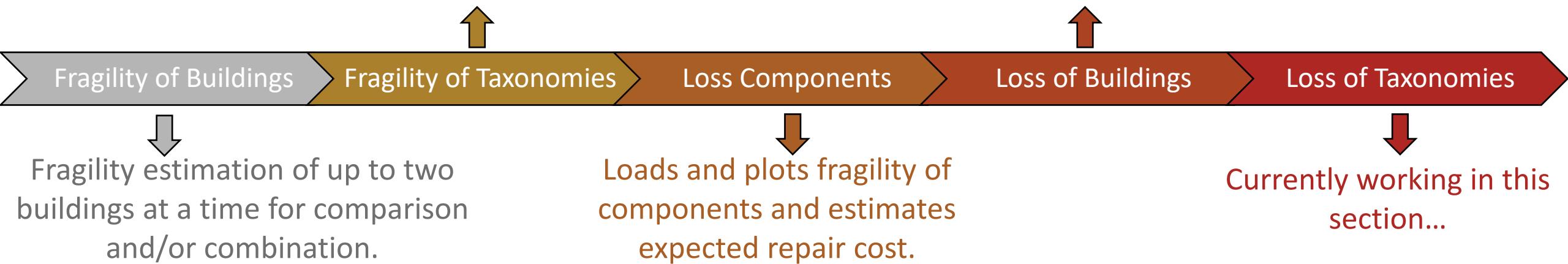


FUNDAMENTALS: FRAGILITY AND VULNERABILITY

V-FAST includes:

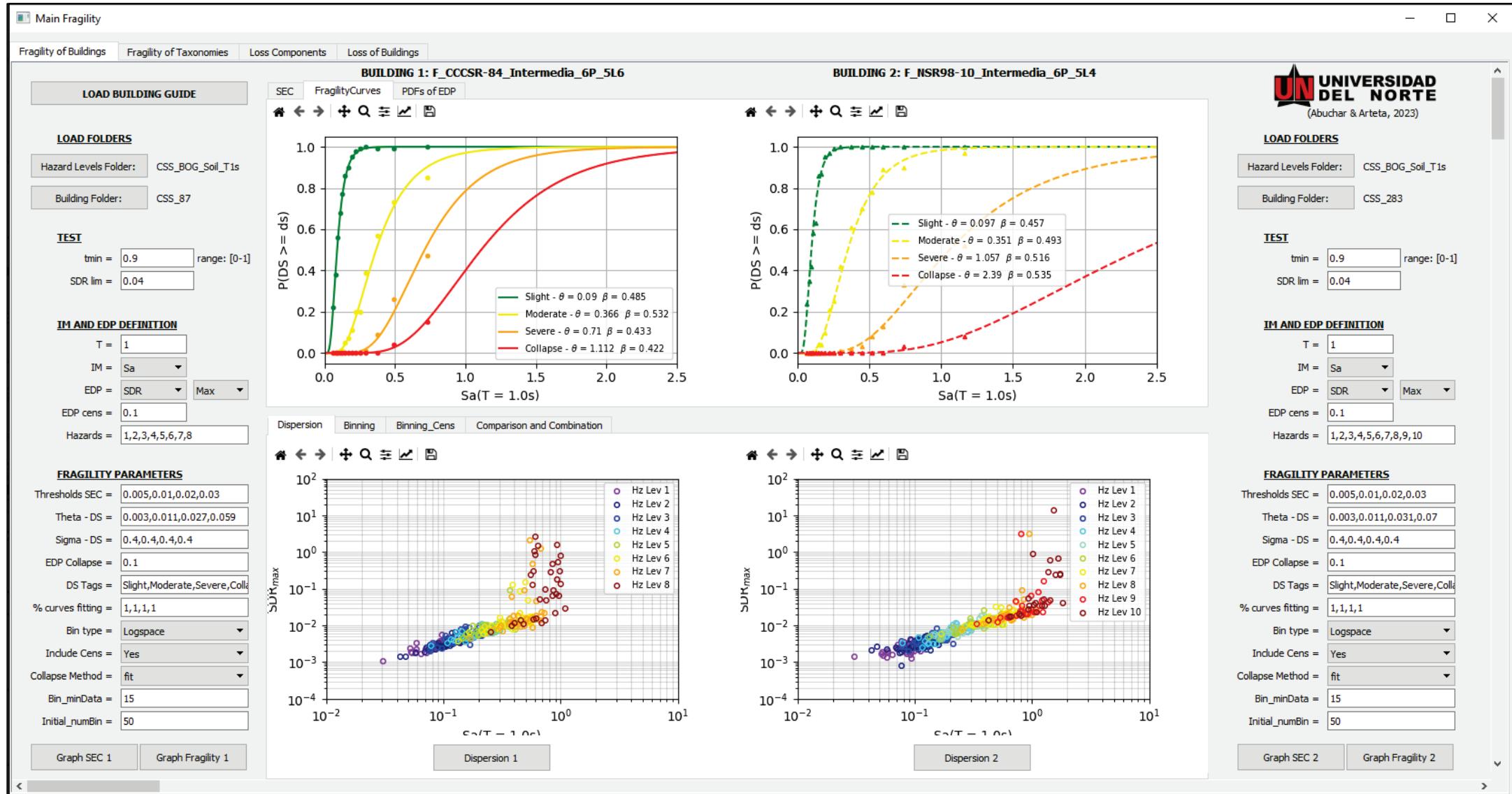
Taxonomy fragility estimations of buildings of the same height and construction typology.

Estimation of expected losses by story and by building.



V-FAST

V-FAST: FRAGILITY OF BUILDINGS





Yielding

$$\theta_y = \phi_y \frac{L_s}{3} + 0.0025 + a_{sl} \frac{0.25 \epsilon_y d_b f_y}{(d - d') \sqrt{f'_c}} \longrightarrow \text{Experimental Studies}$$

First-principles

Ultimate Capacity

$$\theta_u (\%) = \alpha_{st} \alpha_{cyc} \left(1 + \frac{a_{sl}}{2.3} \right) \left(1 - \frac{a_{wall}}{3} \right) (0.2^v) \left[\frac{\max \left(0.01, \frac{\rho' f'_y}{f'_c} \right) f'_c}{\max \left(0.01, \frac{\rho f_y}{f'_c} \right)} \right]^{0.275} \left(\frac{L_s}{h} \right)^{0.45} 1.1 \left(100 \alpha \rho_{sx} \frac{f_y h}{f'_c} \right) (1.3^{100 \rho_d}) \longrightarrow \text{Experimental Studies}$$

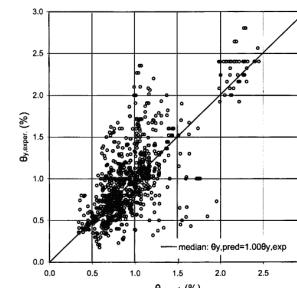
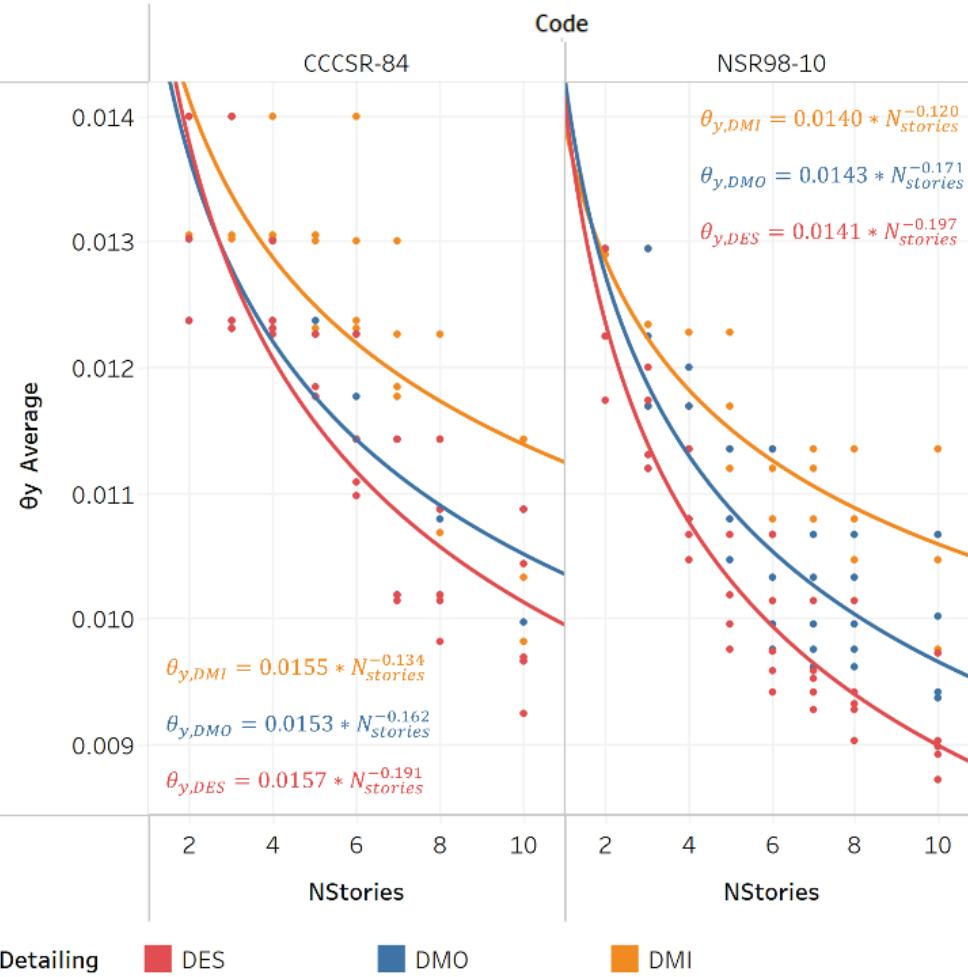
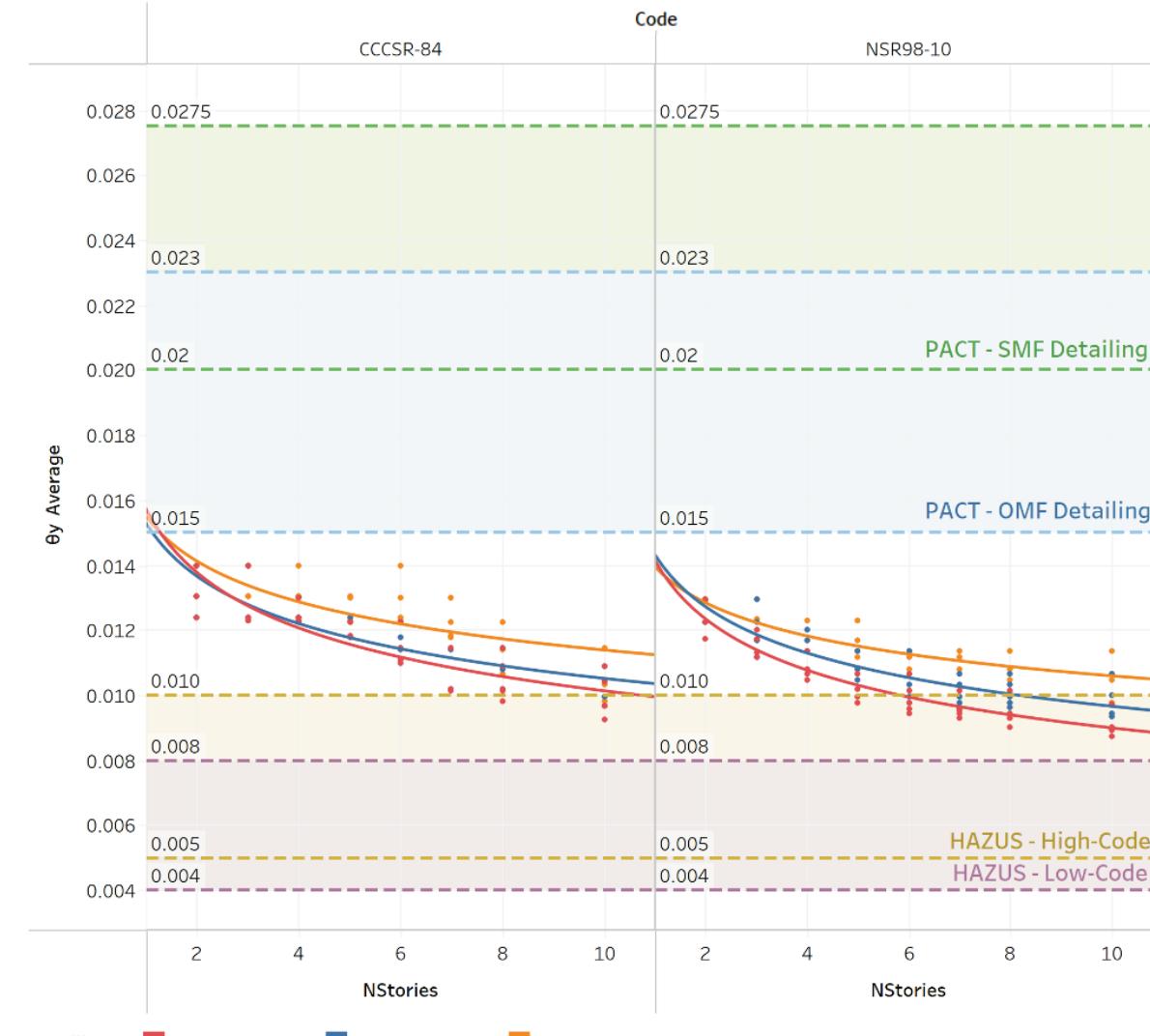


Fig. 2—Comparison of experimental and predicted values of chord rotation (or drift) at yield (963 tests).

Damage states definition for each building height, detailing and code era

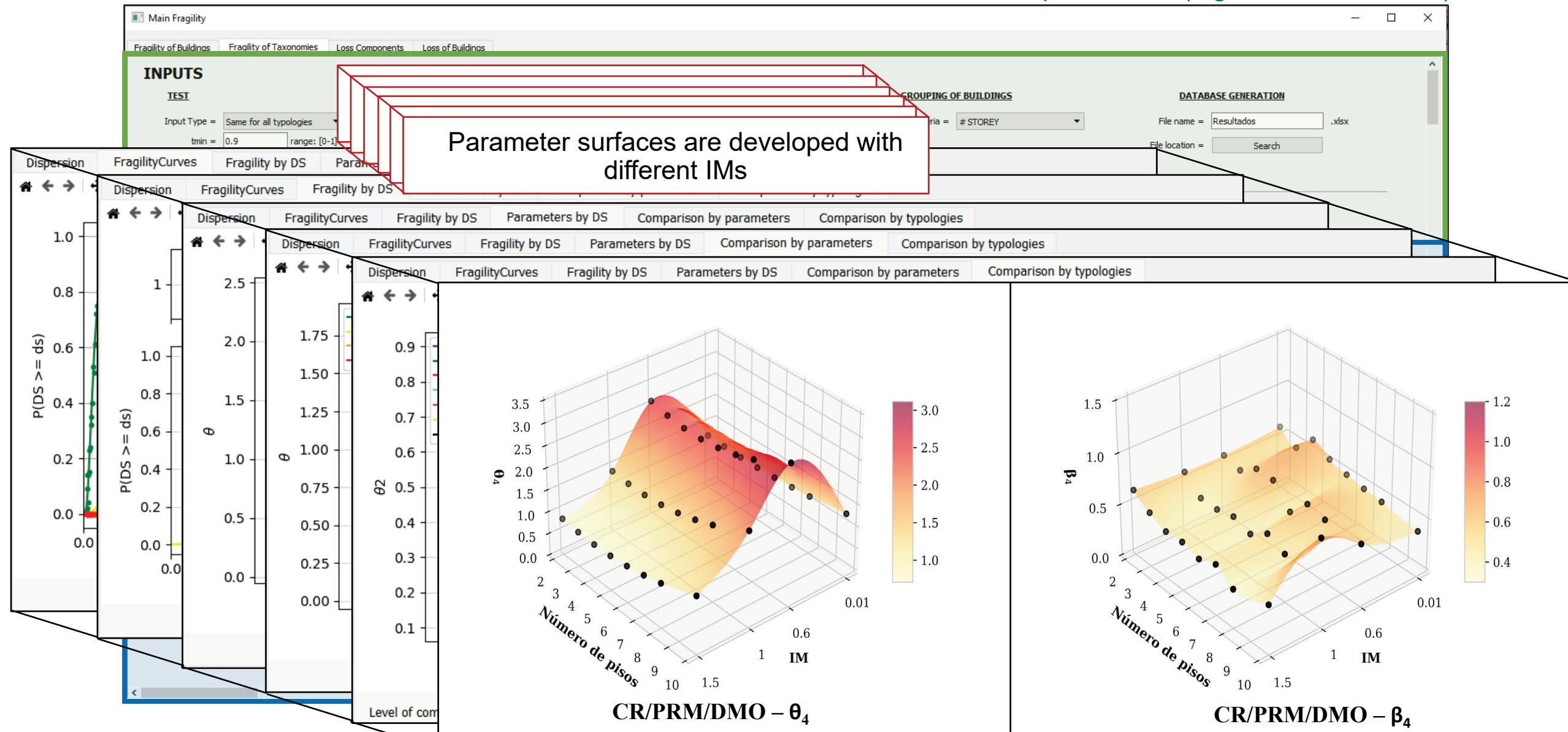


Comparison with PACT and HAZUS



V-FAST: FRAGILITY OF TAXONOMIES

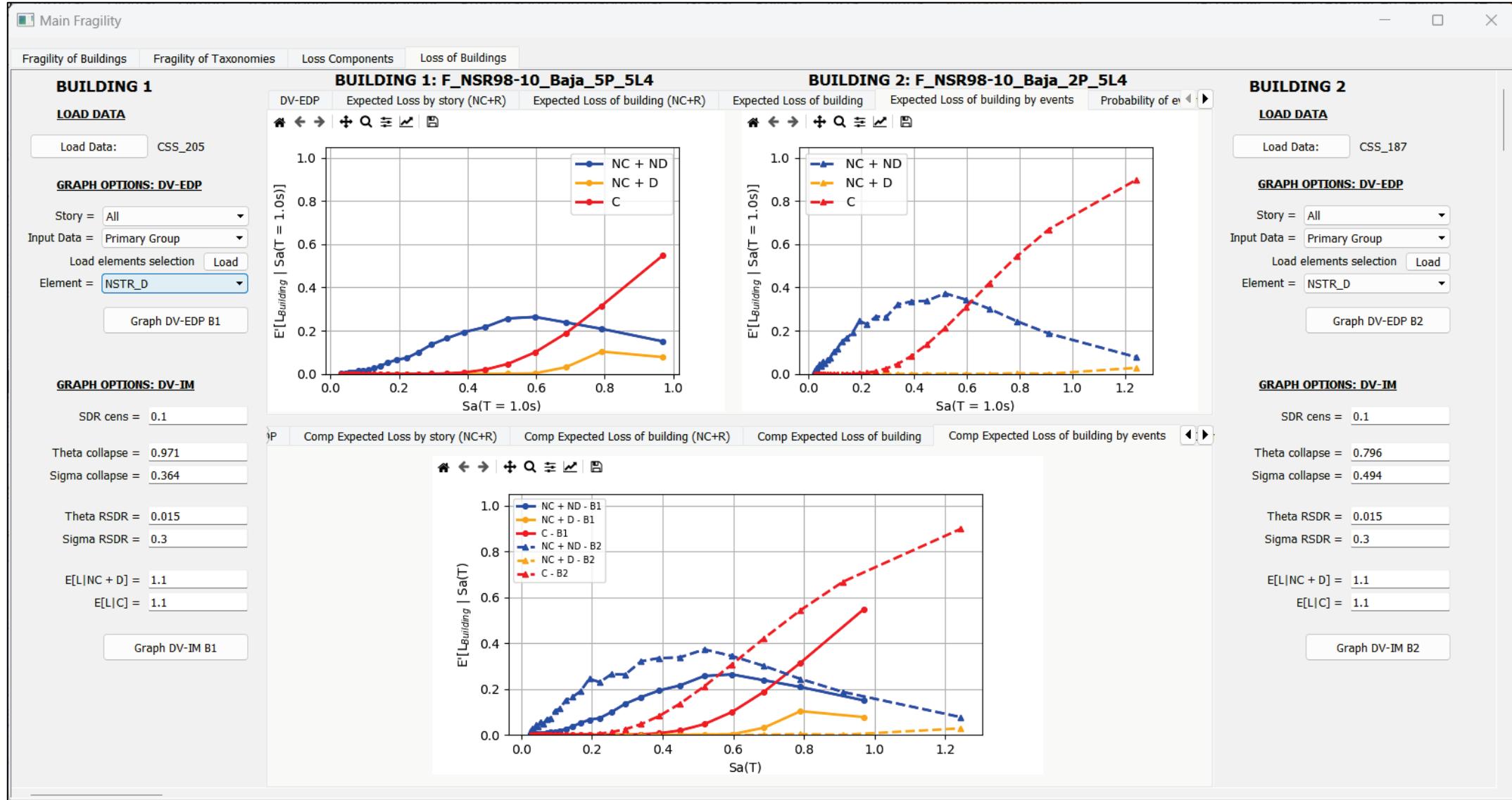
File generation with taxonomy
parameters (e.g., CR/PRM/DMO/3)



V-FAST: LOSS OF BUILDINGS

Expected losses of building by events

Based on Ramirez & Miranda (2009)



CONCLUSIONS

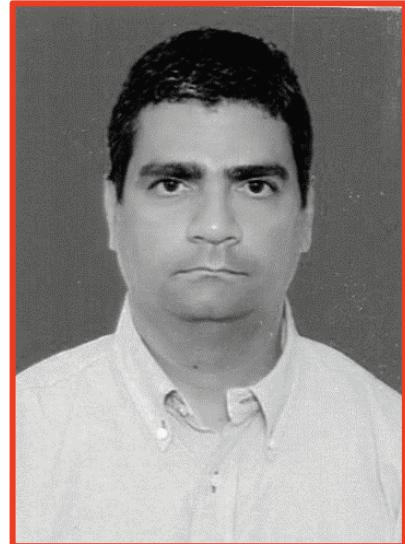
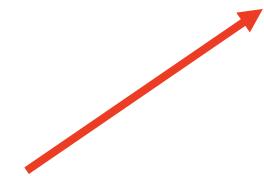
- RC-FIAP and V-FAST are open tools for the practice, research and academia.
- Enables high-resolution modeling of a large number of structural models with ease.
- Enables understanding the consequence of detailing decisions in the seismic risk of RC structural typologies.

Acknowledgements

- SGC - Colombian Geological Survey
- ACOFI - Colombian Association of Engineering Schools



Víctor Ceballos, PhD(s)



bit.ly/RC-FIAP