
Regional Seismic Risk Assessment of a Los Angeles Bridge Network Using a New Generation of Fragility Functions

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Project Team



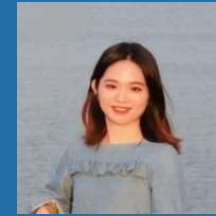
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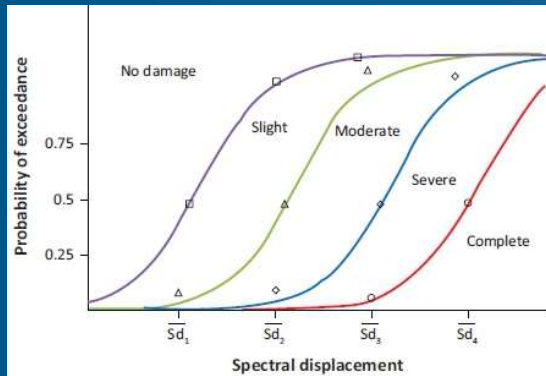
Core Team

Collaborators

Bridge Vulnerability Characterization for Regional Risk Assessment

1st Generation

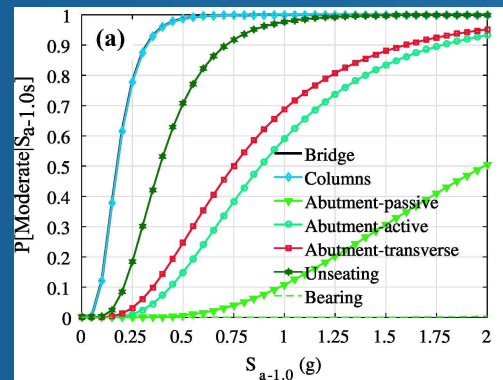
- System-Level Fragilities
- “Low” Resolution Bridge Definition
- Limited Uncertainty Consideration



<https://scielo.org.za/img/revistas/jamba/v10n1/09f06.jpg>

2nd Generation

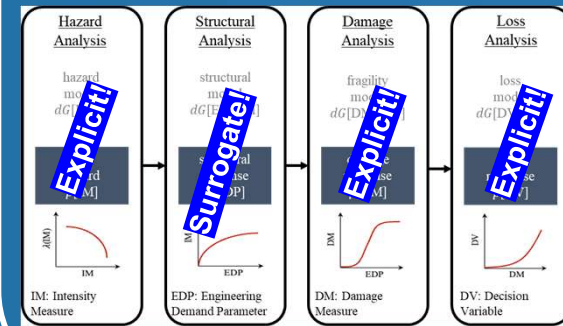
- “Pre-developed” component-level fragilities
- “Moderate” Resolution Bridge Definition
- Enhanced Uncertainty Consideration



Mangalathu et al. 2017

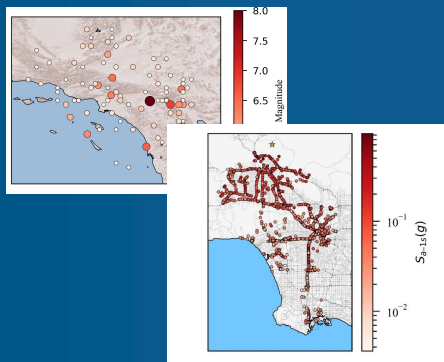
3rd Generation

- Multi-Fidelity (Explicit + Surrogate) Structural Modeling and Performance Assessment
- Multi-Resolution Bridge Definition
- Comprehensive Uncertainty Quantification

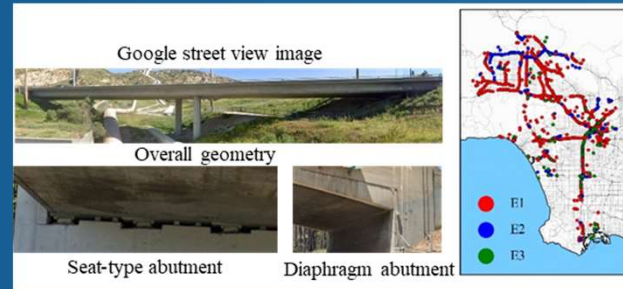


Increasing Fidelity and Resolution + Improved Uncertainty Quantification

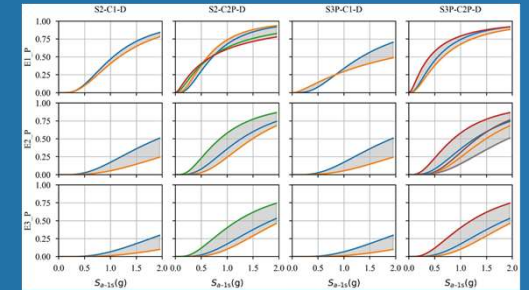
Project Overview



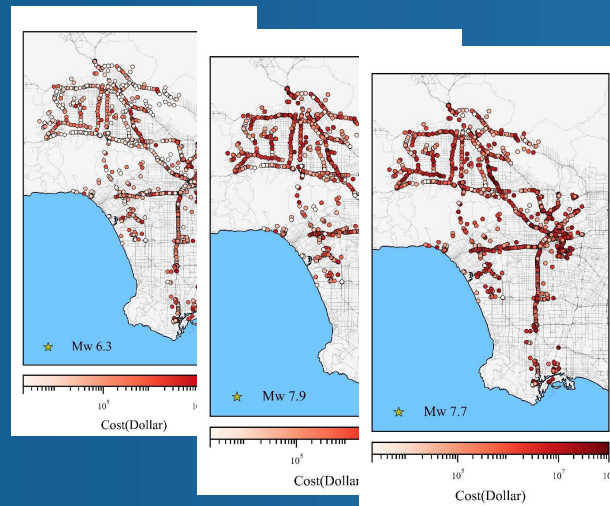
1. Regional Seismic Hazard Characterization



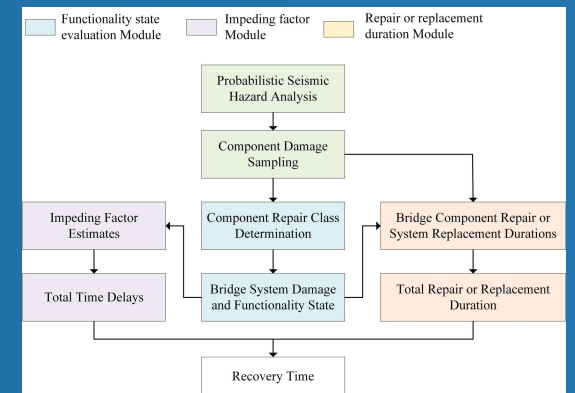
2. Inventory Capture and Group Classification



3. Component-Fragility Definition and Assignment



5. Regional Seismic Impact Assessment



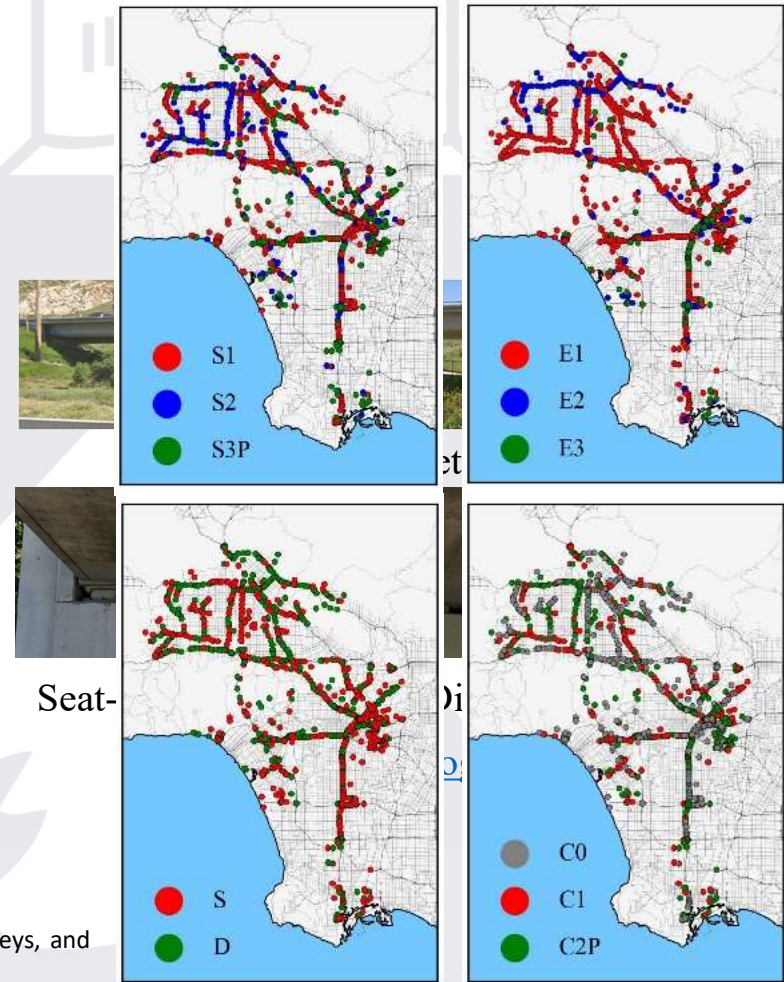
4. Development of Bridge-Specific Repair Cost and Functional Recovery Model

Bridge grouping

Table 1. Nomenclature used for grouping LA bridges.

| Parameters | Design attributes | Nomenclature |
|---------------|------------------------------------|--------------|
| Design era | Era 1 (pre 1970) | E1 |
| | Era 2 (1971-1990) | E2 |
| | Era 3 (post 1991) | E3 |
| Span number | Single span | S1 |
| | Two spans | S2 |
| | More than two spans | S3P |
| Bent type | No column bent (i.e., Single span) | C0 |
| | Single column bent | C1 |
| | Multiple columns bent | C2P |
| Abutment type | Diaphragm | D |
| | Seat type | S |

- Design era represents a change in seismic design philosophy among bridges
- The number of spans and columns in bent has shown evident influences on bridge performance
- Bridges with seat-type abutments have extra components or damage scenarios, such as bearings, shear keys, and span unseating.



Synthesis of the next-generation fragility models

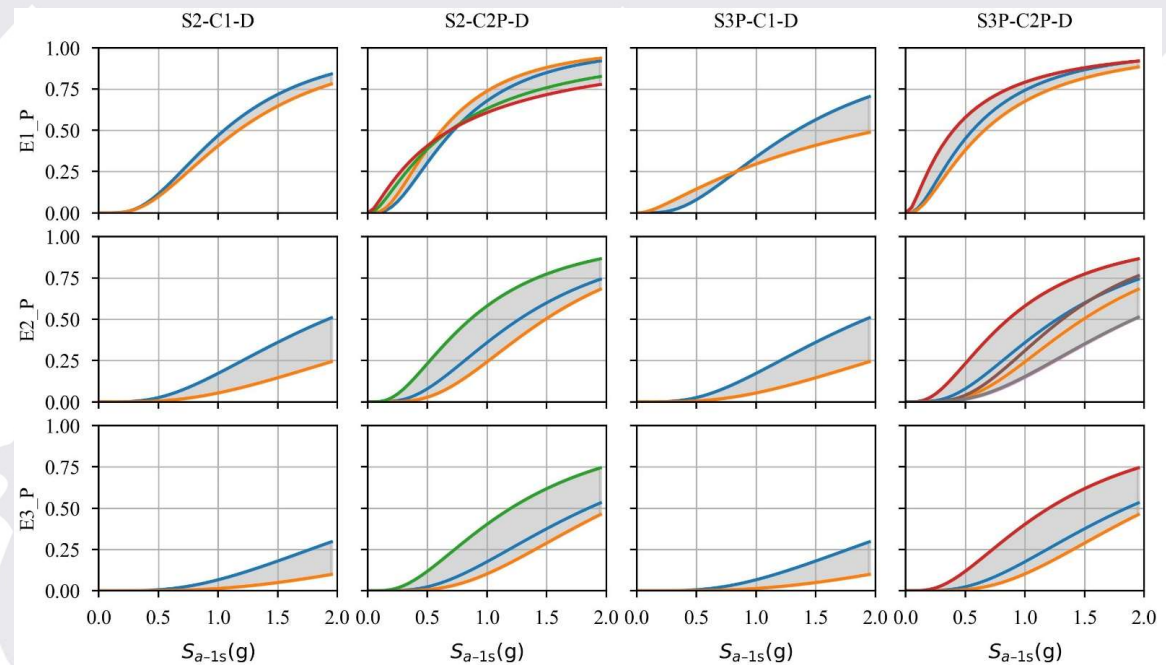
HAZUS Model:

- Small number of classes (e.g., pre- and post-1975 as two design eras)
- Neglecting the effect of abutment type
- Developed using a 2D simplified method
- Failing to address uncertainties in geometric and material attributes
- System-level fragility models

New-Generation Fragility Models:

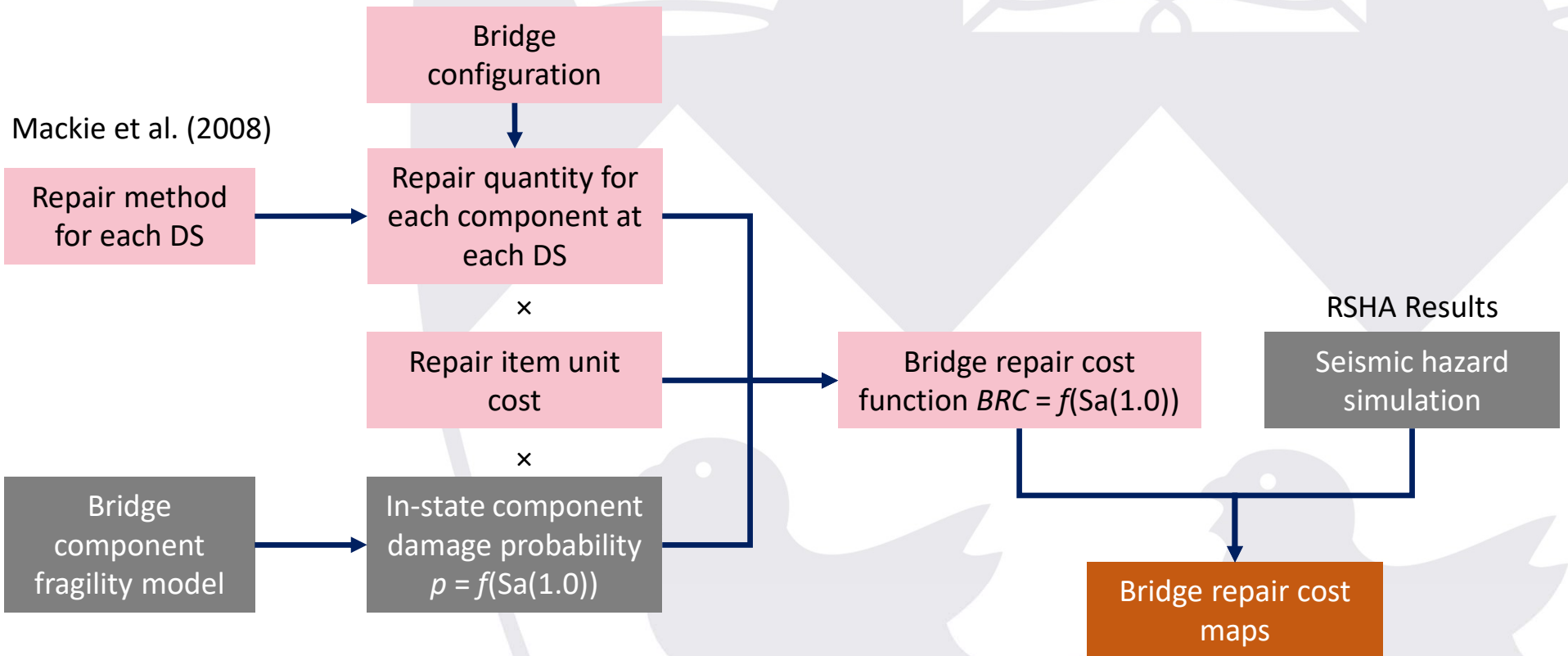
- Larger number of classes (~25)
- Effect of abutment type considered
- Developed using advanced finite element models
- Capturing uncertainties in geometric and material attributes of California bridges
- Component-level fragility models

Column fragility, diaphragm bridge, complete damage state



(Mangalathu 2017; Mangalathu et al. 2017; Soleimani 2017; Zheng 2021; Mangalathu et al. 2018; Jeon et al. 2019; Soleimani et al. 2017)

Bridge repair cost modeling



Mackie KR, Wong JM, Stojadinovic' B. Integrated probabilistic performance-based evaluation of benchmark reinforced concrete bridges. Pacific Earthquake Engineering Research Center; 2008.

Issues in Mackie's model and modifications

1. Some repair quantity items are bridge geometry dependent (e.g., Repair minor column spalls = $25\% \times (\text{column surface area}) \times (\text{cover} + 1'')$). Mackie et al. (2008) only considers a testbed bridge (Type 1A), which has one fixed configuration (a two-span seat-type bridge) and a specific geometry.

Solution: Identify component geometric parameters (column, abutment wall, wing wall, shear key, pile foundation, deck depth, etc.) for each bridge.

- NBI database
- PhD thesis from Zheng (2021)
- Caltrans Engineering Manuals (<https://dot.ca.gov/programs/engineering-services/manuals>)

Issues in Mackie's model and modifications

2. Mackie et al. (2008) has a different damage state (DS) definition (not matching HAZUS's definition and those used for the new generation fragility models); it does not consider repair actions for span unseating

Solution: Remapping DS definition from Mackie's model to the HAZUS definition. Develop repair cost models (repair method, quantify, unit cost) for missing DS and for span unseating at four DSs.

| Repair strategy | Probability (%) of selecting different repair strategies | | | |
|--|--|-----|-----|------|
| | S | M | E | C |
| Jack bridge into place ^d | 15% | 42% | 41% | 0% |
| Do nothing | 77% | 12% | 0% | 0% |
| Replace bridge bearings ^a | 8% | 46% | 0% | 0% |
| Replace bridge deck ^c and bearings ^a | 0% | 0% | 59% | 100% |

Span Unseating

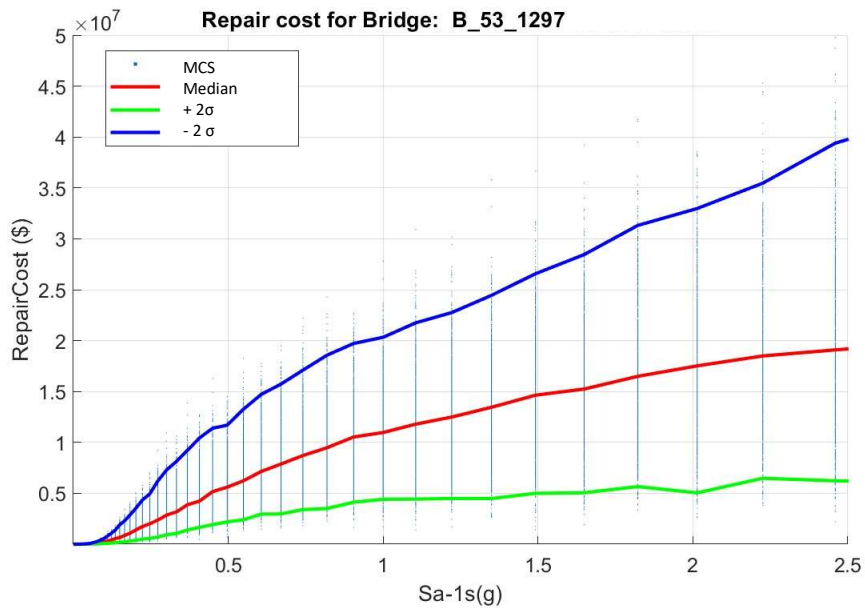
Padgett, J. E., and R. DesRoches. 2007. "Bridge functionality relationships for improved seismic risk assessment of transportation networks." *Earthquake Spectra* 23 (1): 115–130. <https://doi.org/10.1193/1.2431209>.

Issues in Mackie's model and modifications

3. Repair item unit cost in Mackie et al. (2008) is based on 2007 data

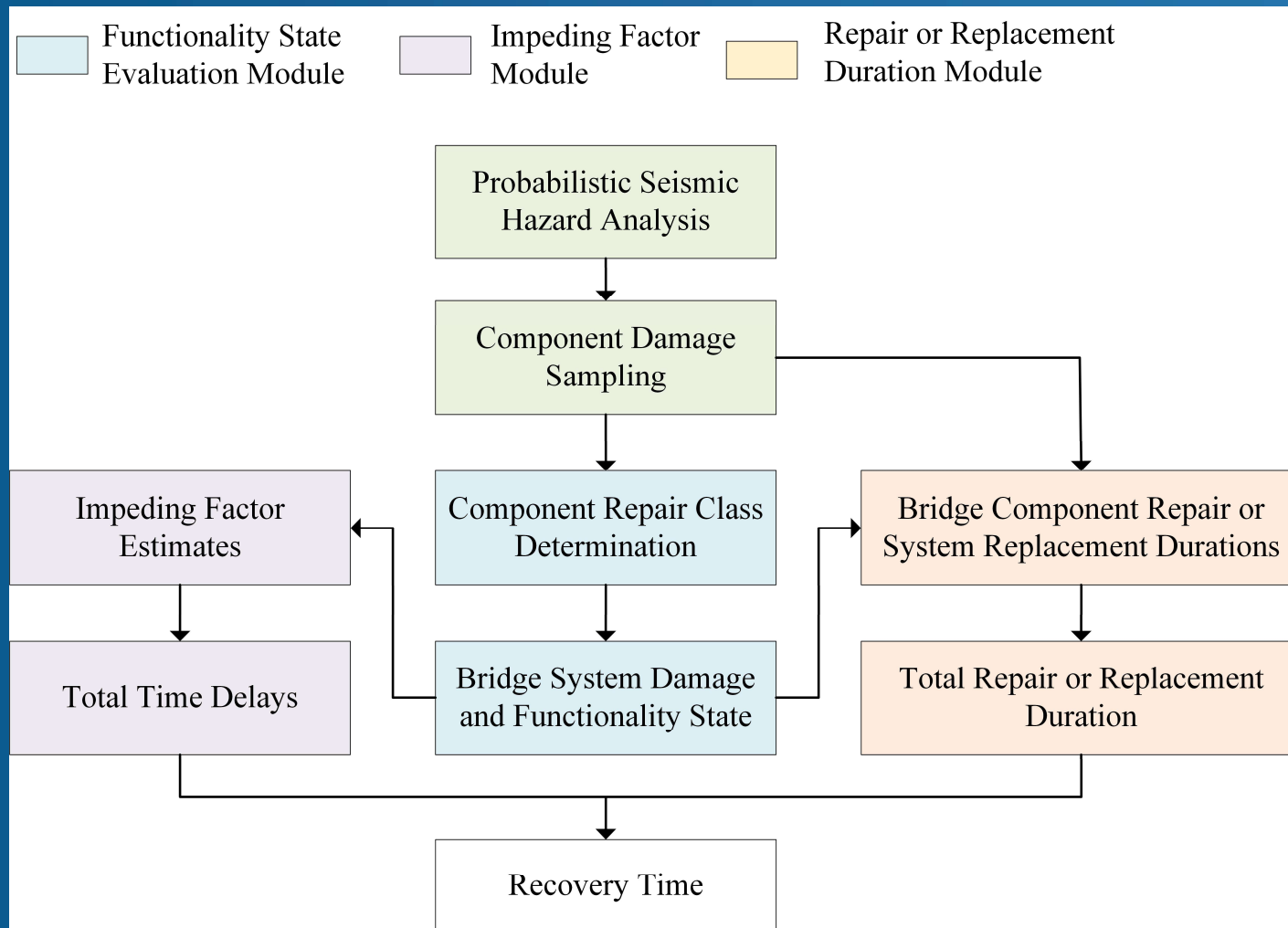
Solution: Replace it with the Caltrans Contract Cost Data (2023) (<https://sv08data.dot.ca.gov/contractcost/>)

Bridge repair cost function



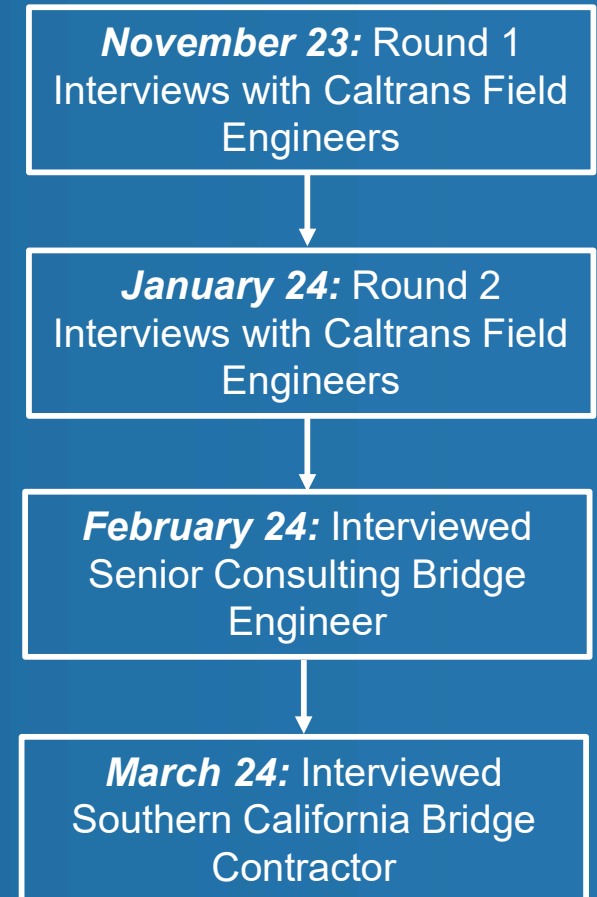
- Cost function development: For each bridge, Monte Carlo simulation (MCS) to capture uncertainties in in-state damage probabilities (i.e., from fragility curves) and unit costs for each repair item.
- Cost function utilization: at each Sa-1 level, fit the bridge repair cost as a normal distribution function and generate the bridge cost through normal distribution sampling.

Bridge-Specific Post-Earthquake Functional Recovery Model

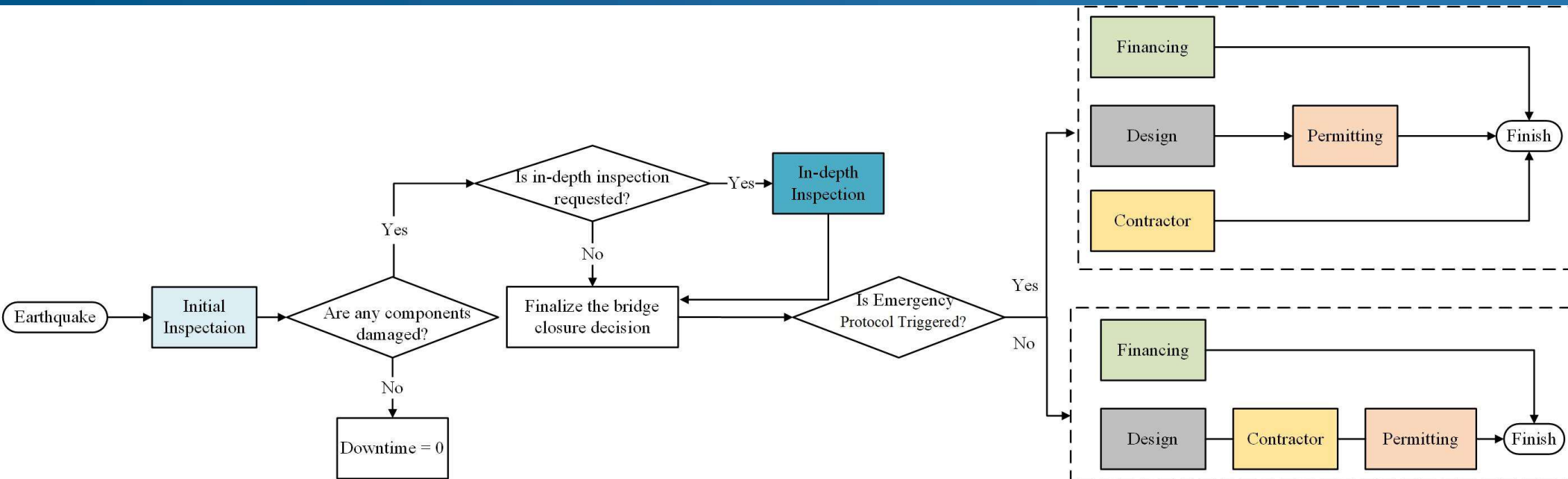


Framework Supported by Information and Data from Expert Elicitation

- Expert elicitation sought from:
 - Caltrans Field Engineers
 - Senior Consulting Bridge Engineer (> 30 years experience)
 - Southern California Bridge Contractor
- Information and data include:
 - Type sequencing and duration of impeding factors
 - Bridge closure and reopening decision making
 - Sequencing and duration for component-level repairs
 - Replacement durations



Impeding Factors and Sequencing**



** Based on discussions with Caltrans i.e., will need to be modified for Non-Caltrans bridges

Time Delays Associated with Impeding Factors

| Impeding Factor | System Damage State | | | | | | | |
|------------------------|---------------------|---------|----------|----------|---------------|----------|----------|----------|
| | Slight | | Moderate | | Extensive | | Complete | |
| | Min | Max | Min | Max | Min | Max | Min | Max |
| Initial Inspection | 0 to 6 hours | | | | | | | |
| In-depth Inspection | 3 days | 7 days | 3 days | 7 days | 2 hours | 36 hours | 0 | |
| Financing | 6 months | 2 years | 0 | 6 months | 0 | | | |
| Design | 0 | | 1 month | 2 months | 1 week | 2 months | 2 weeks | 2 months |
| Permitting | 1.5 to 3 months | | | | 1 to 7 days | | | |
| Contractor Acquisition | 1 year | 2 years | 3 months | 6 months | 6 to 48 hours | | | |

** Based on discussions with Caltrans, a bridge engineer and a bridge builder

Functionality State Definitions

| Functionality state | Applicable Phase(s) |
|---|--|
| FS1: Fully Functional | Initial response and reopening phases |
| FS2: Minor-to-Moderate Lane Closure | |
| FS3: Moderate-to-Extensive Lane Closure | |
| FS4: Reopen with Weight Restriction | Reopening phase |
| FS5: Reopen with Minor Lane Closure | |
| FS6: Reopen with Weight Restrictions and Minor Lane Closure | |
| FS7: Complete Closure | Initial response and reopening phases |

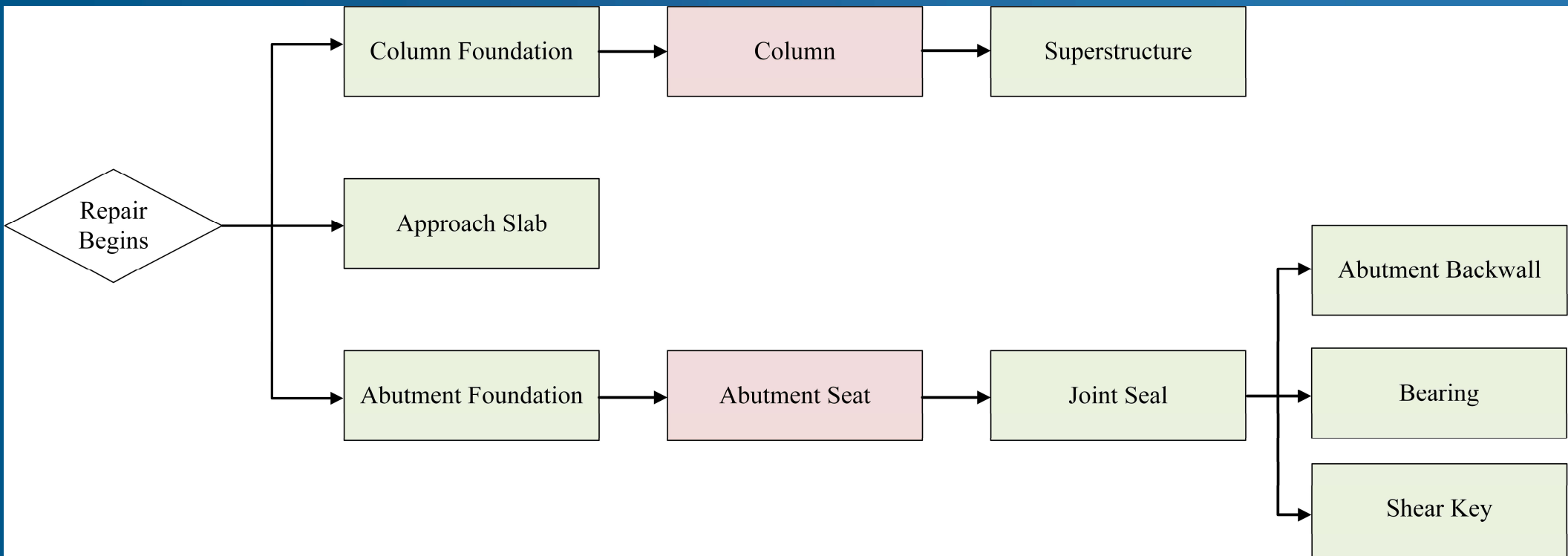
* The “fully repaired” state is one where all components are intact or have been repaired and the bridge is fully functional. Since it does not represent a change in functionality relative to FS1, it is not listed in the table.

Component Repair Class Definitions

| Component Type | Damage State | | | | |
|---------------------|--------------|--|--|--|----------|
| | No Damage | Slight | Moderate | Extensive | Complete |
| Column | RC1 | RC2 for $N_c \geq 1$ & $F_c < 50\%$ RC3 for $F_c \geq 50\%$ | RC3 for $N_c \geq 1$ & $F_c < 50\%$ RC4 for $F_c \geq 50\%$ | RC4 for $N_c \geq 1$ & $F_c < 50\%$ RC5 for $F_c \geq 50\%$ | RC5 |
| Abutment seat | RC1 | RC2 for $N_c = 1$ RC3 for $N_c = 2$ | RC3 for $N_c = 1$ RC4 for $N_c = 2$ | RC4 for $N_c = 1$ RC5 for $N_c = 2$ | RC5 |
| Superstructure | RC1 | RC2 | RC2 for $N_c \geq 1$ & $F_c < 50\%$ RC3 for $F_c \geq 50\%$ | / | |
| Column foundation | RC1 | RC2 | RC2 for $N_c \geq 1$ & $F_c < 50\%$ RC3 for $F_c \geq 50\%$ | | |
| Abutment foundation | RC1 | RC2 | RC2 for $N_c = 1$ RC3 for $N_c = 2$ | | |
| Bearing | RC1 | RC2 | RC2 for $N_c = 1$ RC3 for $N_c = 2$ | | |
| Shear key | RC1 | RC2 | RC2 for $N_c = 1$ RC3 for $N_c = 2$ | | |
| Abutment backwall | RC1 | RC2 | RC2 for $N_c = 1$ RC3 for $N_c = 2$ | | |
| Approach slab | RC1 | RC2 | RC2 for $N_c = 1$ RC3 for $N_c = 2$ | | |
| Joint seal | RC1 | RC2 | RC2 for $N_c = 1$ RC3 for $N_c = 2$ | | |

* N_c denotes the number of damaged components, and F_c denotes the fraction of damaged components.

Component Repair Sequencing (Seat-Type Bridge)**



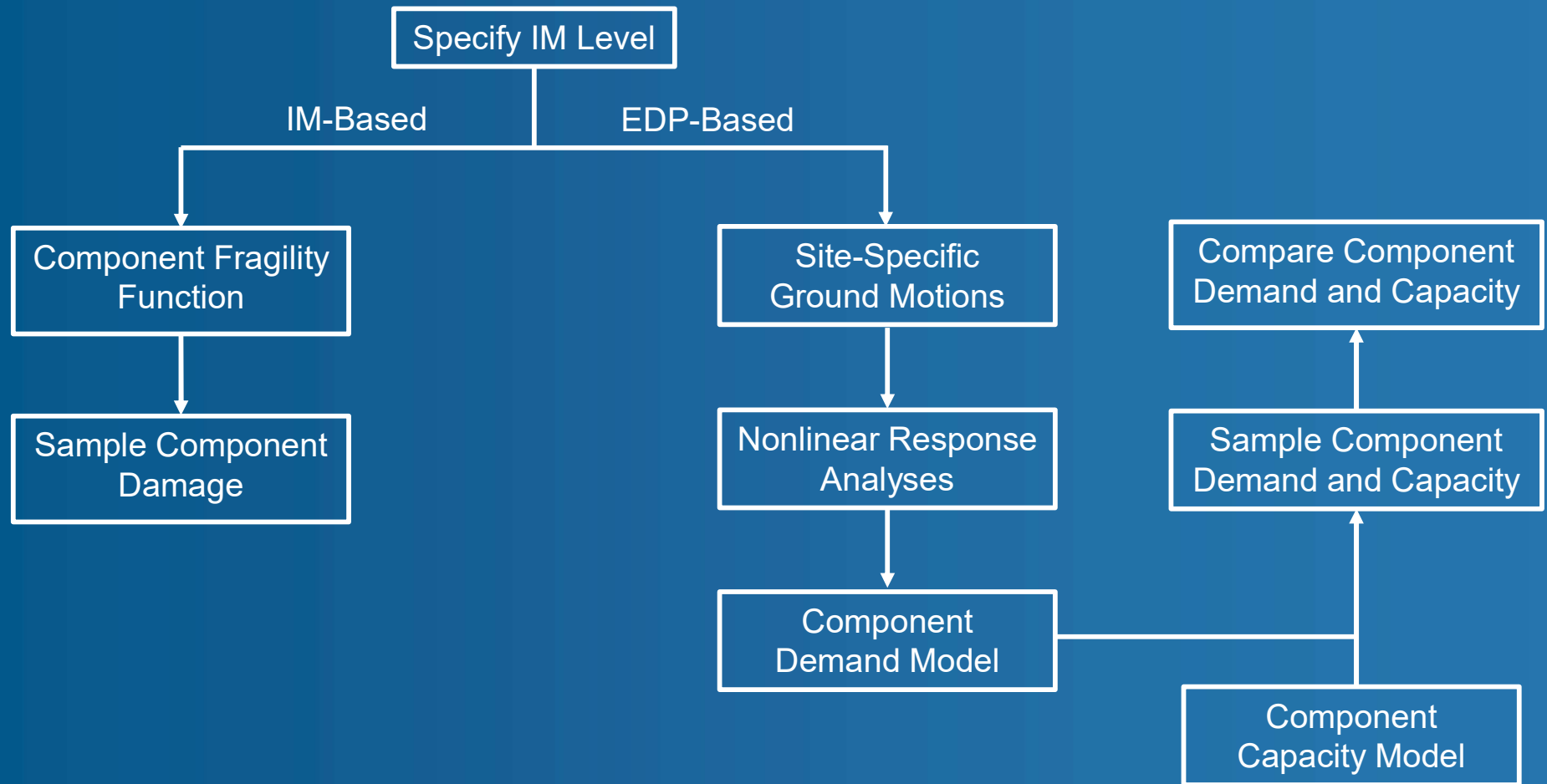
** Based on discussions with Caltrans, a bridge engineer and a bridge builder

Component Repair Durations (Per Unit)**

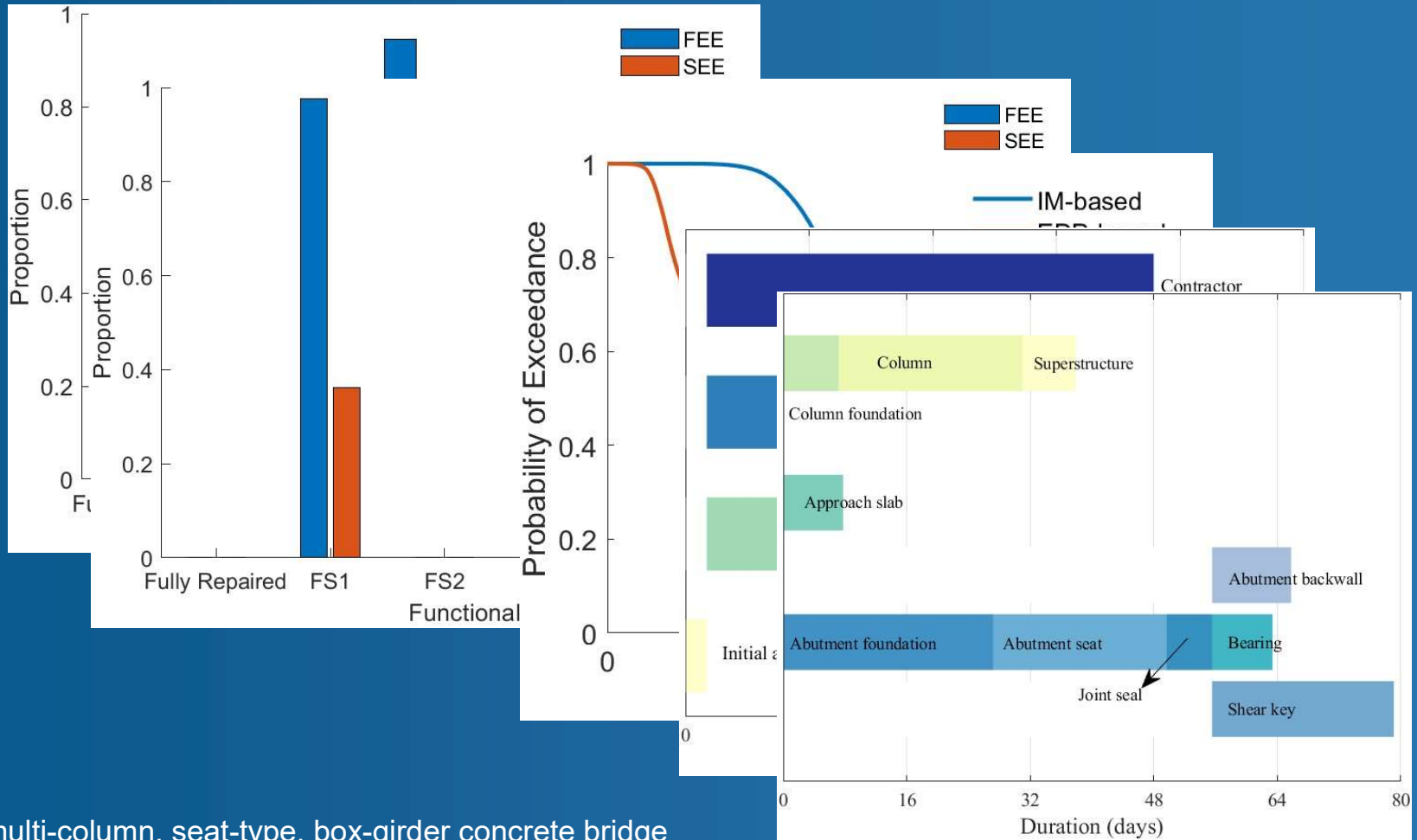
| Component Type | Median repair duration (days) | | | | | | Required No. of Workers | | | | | |
|---------------------|-------------------------------|-----|----------|-----|-----------|-----|-------------------------|-----|----------|-----|-----------|-----|
| | Slight | | Moderate | | Extensive | | Slight | | Moderate | | Extensive | |
| | Min | Max | Min | Max | Min | Max | Min | Max | Min | Max | Min | Max |
| Column | 3 | 5 | 3 | 10 | 33.5 | 46 | 1 | 6 | 2 | 6 | 3 | 6 |
| Abutment seat | 3 | 5 | 4.5 | 15 | 42 | 65 | 1 | 6 | 3 | 6 | 3 | 6 |
| Superstructure | 3 | 5 | 3 | 10 | / | / | 4 | 6 | 4 | 6 | / | / |
| Column foundation | 6 | 17 | 6 | 17 | | | 3 | 6 | 3 | 6 | | |
| Abutment foundation | 6 | 17 | 6 | 17 | | | 3 | 6 | 3 | 6 | | |
| Bearing | 1.5 | 6 | 1.5 | 6 | | | 2 | 6 | 2 | 6 | | |
| Shear key | 3 | 5 | 12 | 25 | | | 1 | 6 | 3 | 6 | | |
| Abutment backwall | 4 | 9 | 12.5 | 26 | | | 2 | 6 | 2 | 6 | | |
| Approach slab | 1 | 4 | 2 | 15 | | | 4 | 7 | 4 | 7 | | |
| Joint seal | 2 | 4 | 2 | 4 | | | 4 | 6 | 4 | 6 | | |

** Based on discussions with Caltrans, a bridge engineer and a bridge builder

IM-Based versus EDP-Based Component Damage Assessment



Sample Results for Hypothetical Bridge**



** For two-span, multi-column, seat-type, box-girder concrete bridge

Summary and Next Steps

- At a high level, this project seeks to advance bridge vulnerability characterization and assessment for regional seismic risk and resilience assessments.
- “Year 1” focused on
 - Bridge inventory (for Los Angeles) capture and group classification
 - Component-fragility definitions and assignment
 - Developing and improving methods for bridge-specific performance quantification based on post-earthquake repair cost and functional recovery.
- “Year 2” will focus on
 - Regional hazard characterization.
 - Network level performance assessment
 - Miscellaneous topics (e.g., uncertainty quantification, surrogate modeling, improving workflow efficiency, model integration into SimCenter tools)

Acknowledgements

- UCLA student funded by the PEER-TSRP Topic 2: Forward Uncertainty Quantification.
- McGill student funded by the Natural Sciences and Engineering Research Council (NSERC) of Canada.



The End

