

# Modeling Vertical Free-Field Motion for SSI Analysis Consistent with Vertical Design Motion Development



## DOE/PEER/UNR Workshop

*International Workshop on Large-Scale Shake Table Testing for the Assessment of Soil-Foundation-Structure System Response for Seismic Safety of DOE Nuclear Facilities*



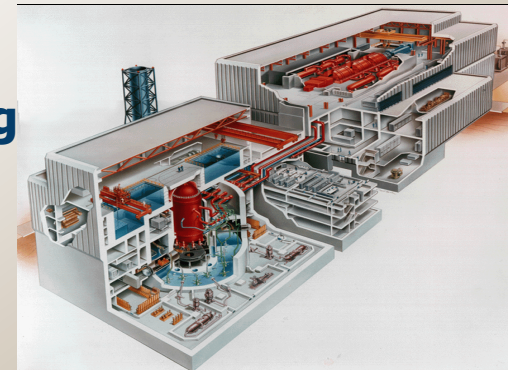
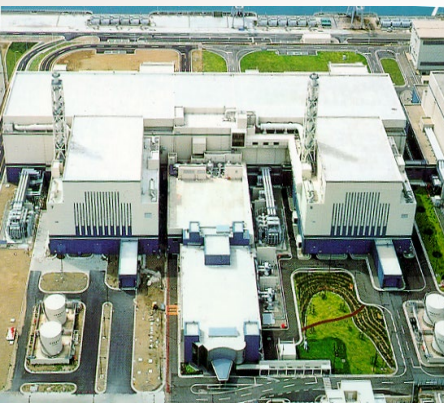
**May 18, 2021**

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Center**

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**EPRI Report**

<https://www.epri.com/#/pages/product/3002011804/>



# Current Practice

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- Development of seismic design motion for NPP application begins with PSHA and follows with a robust site amplification analysis (NUREG/CR 6728) to develop the horizontal design response spectra (GMRS, FIRS)
- For vesical design response spectra development, vertical P-wave analysis is no longer performed. P-wave amplifications are found to be spurious and not consistent with observation
- Instead, applicable V/H spectral ratios are used in practice to develop vertical design spectra. There are few publications outlining the formulation of the V/H ratios
- For horizontal SSI analysis, the horizontal design spectra or associated time histories are used for analysis
- For vertical SSI analysis, vertical P wave is modeled in the free-field
  - ✓ This approach is inconsistent with development of the vertical design spectra
  - ✓ SSI results are overly conservative (ISRS for equipment design)
  - ✓ Results in buoyancy stability issues for plant structures with embedment (shallow and deep embedment)

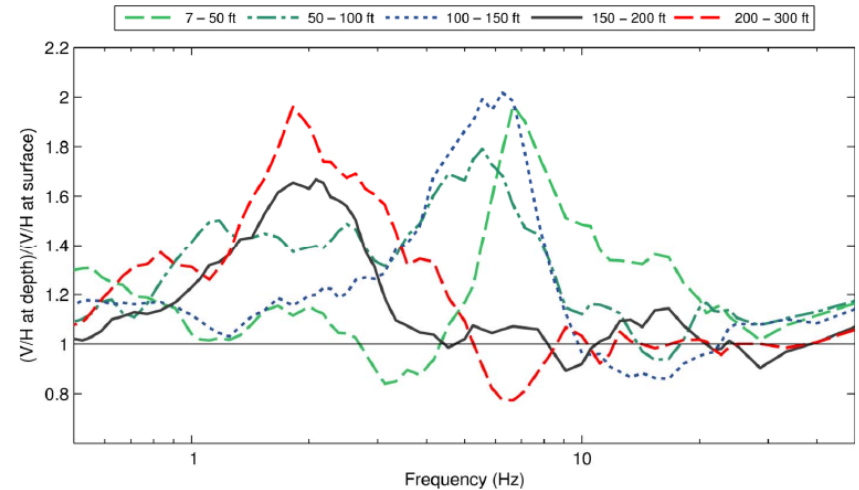
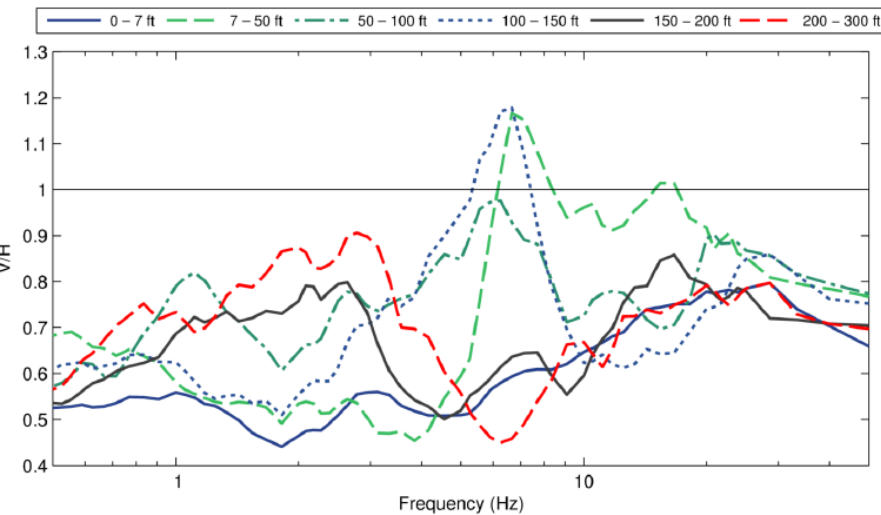


# New Approach

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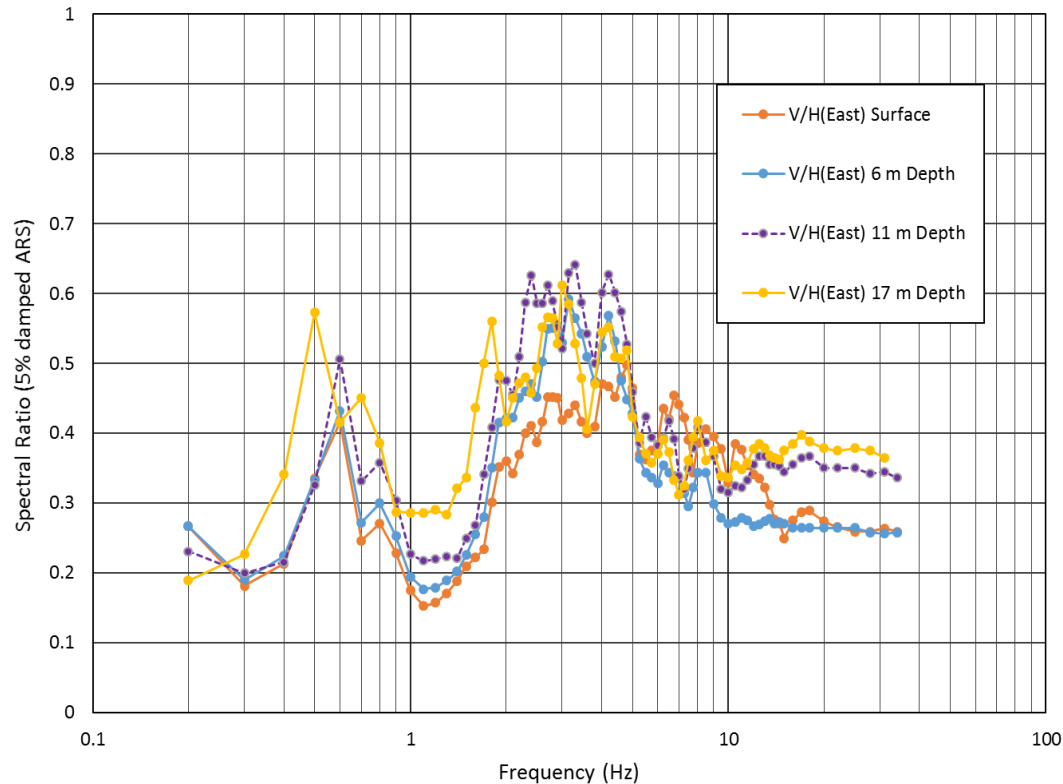
- For SSI analysis of embedded structures using the substructuring methods (SASSI), the free-field motion within the embedment depth of the structures needs to be computed. In other SSI formulation, free-field motion for the full height of the soil column is needed
- In the new approach, the free-field motion at each depth in the model is computed using the horizontal motion at the same depth and applying applicable V/H ratio to get the vertical motion
- This approach is formulated in frequency domain using RVT (random vibration theory) in an iterative process to get the vertical spectra at all depth of interest for SSI analysis
- In this approach, free-field SSI vertical motion is consistent with the approach used for development of the vertical design spectra
- The vertical SSI results are more realistic and are reduced from the results using P-wave analysis

# Depth-dependent Vertical-to-Horizontal (V/H) Ratios of Free-Field Ground Motion Response Spectra for Deeply Embedded nuclear Structures (BNL-107612-2015-R, 2/2015)



(Data from 45 vertical arrays: California, Japan, Alaska, Taiwan)

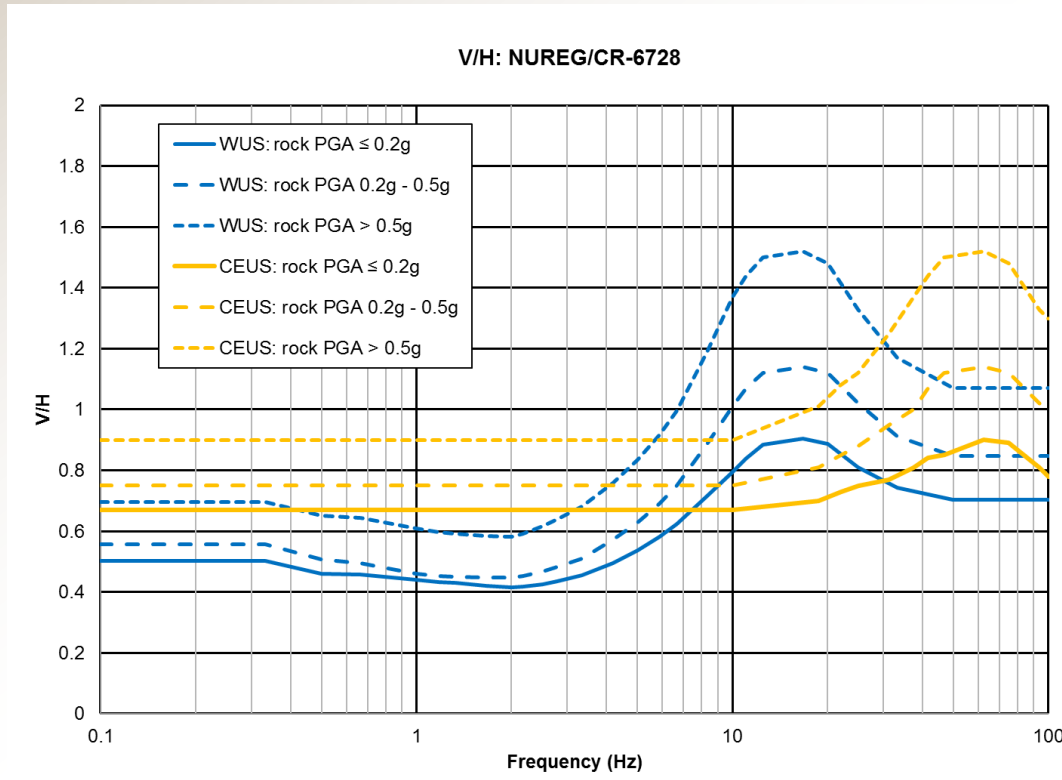
## Lotung SSI Experiment (1980s)



V/H Spectral Ratio from Lotung LSST No. 7 Free-Field records

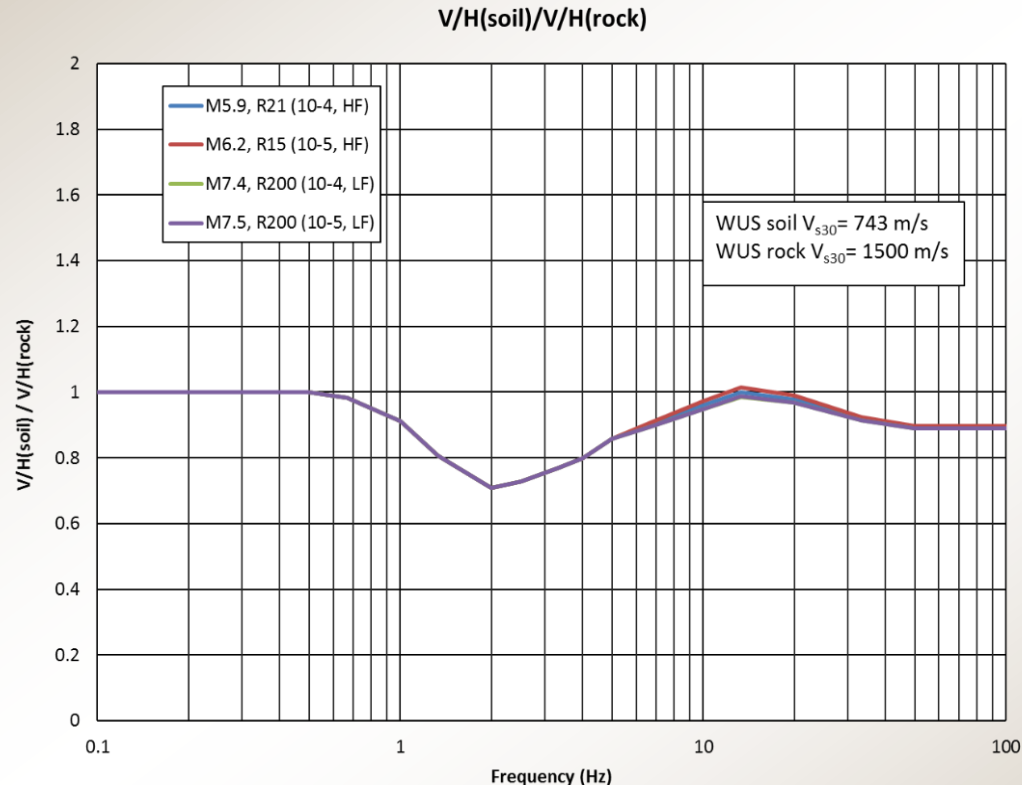


# Commonly Used V/H Ratios



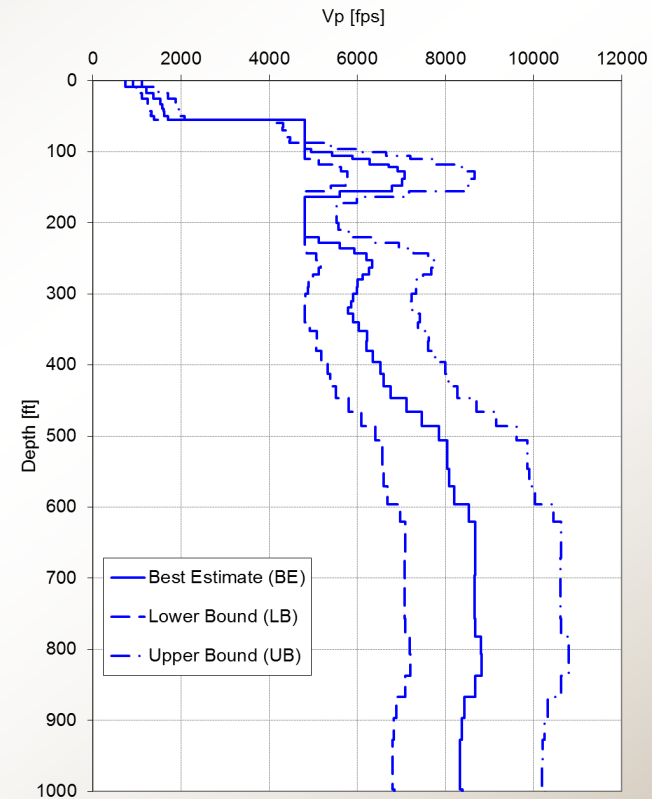
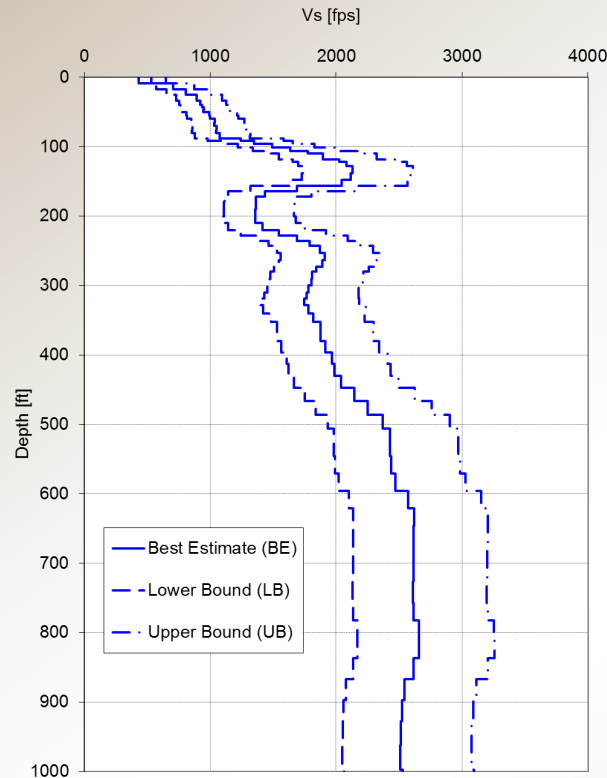
V/H ratios for WUS rock and CEUS Hard Rock Sites,  
NUREG/CR-6728 at 5% spectral damping

# Commonly Used V/H Ratios



Gülerce and Abrahamson  $V/H(\text{WUS,soil}) / V/H(\text{WUS,rock})$  for a suite of controlling magnitudes and distances for  $V_{s30}$  of 743 m/sec and 1,500 m/sec at 5% spectral damping

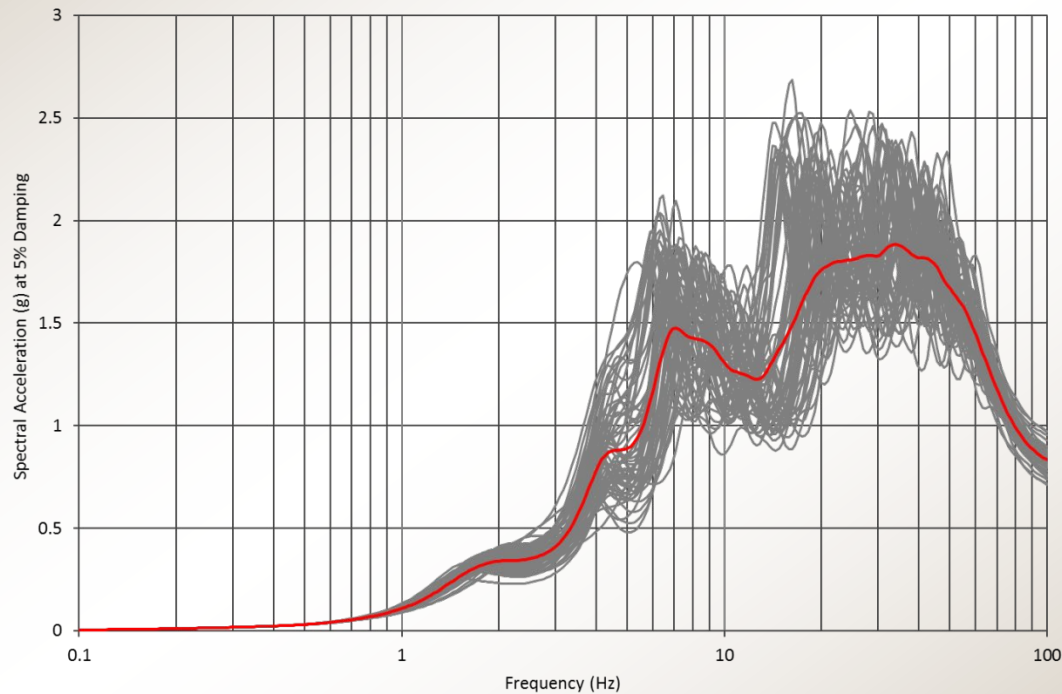
# Anomalies Associated with P-wave Site Amplification



Shear and P-wave Velocity Profiles at a Soil Plant Site in US

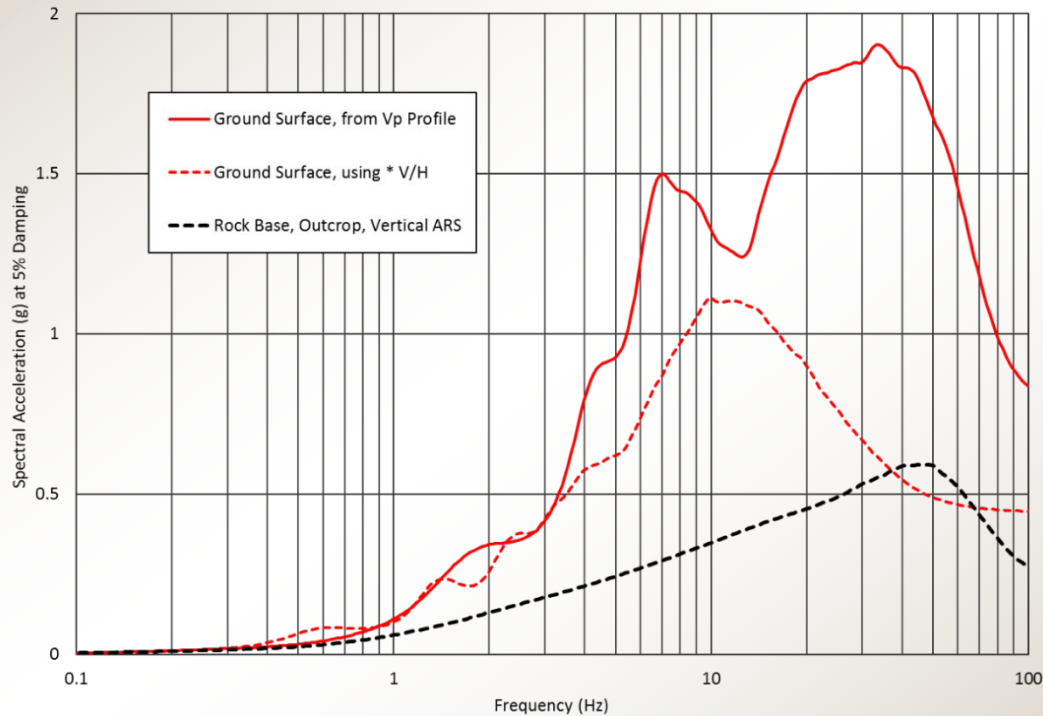


# Anomalies Associated with P-wave Site Amplification



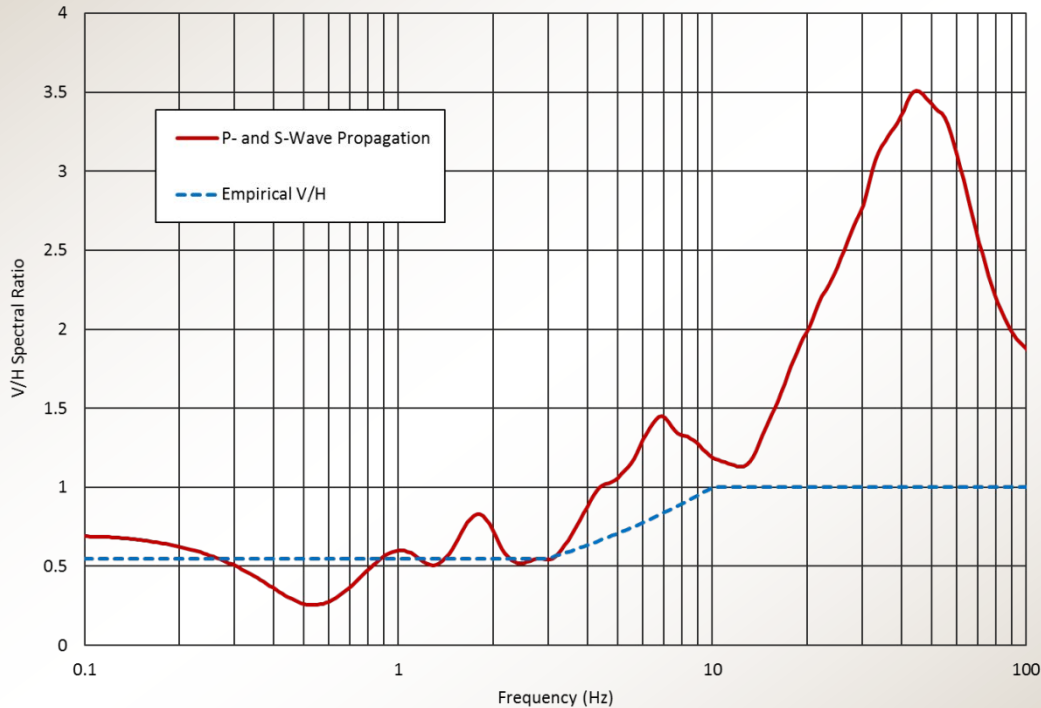
Randomized P-wave Profile

# Anomalies Associated with P-wave Site Amplification



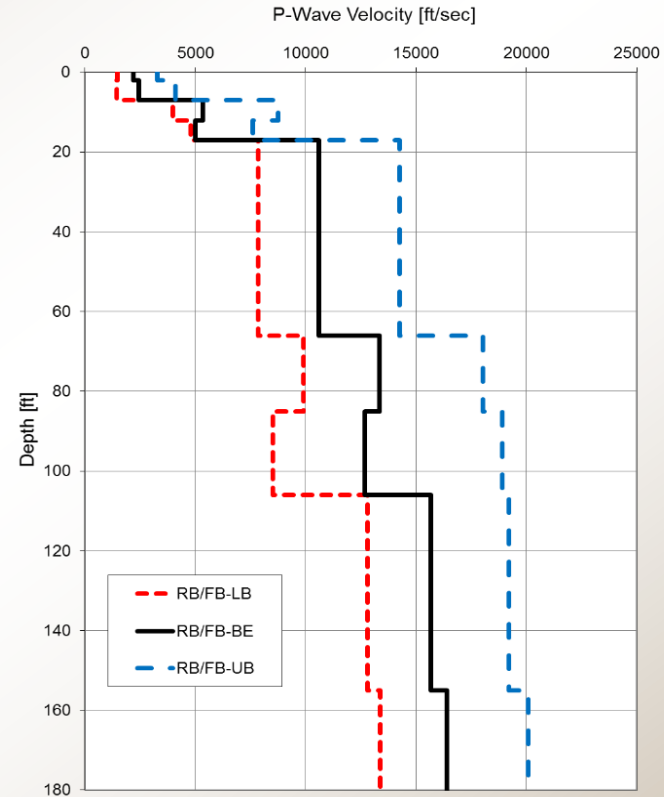
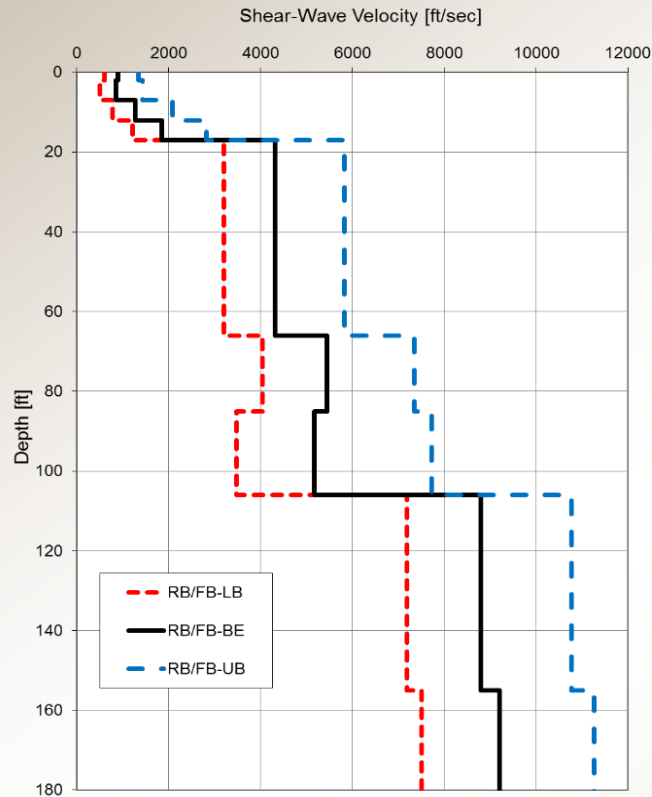
Responses at the Surface

# Anomalies Associated with P-wave Site Amplification



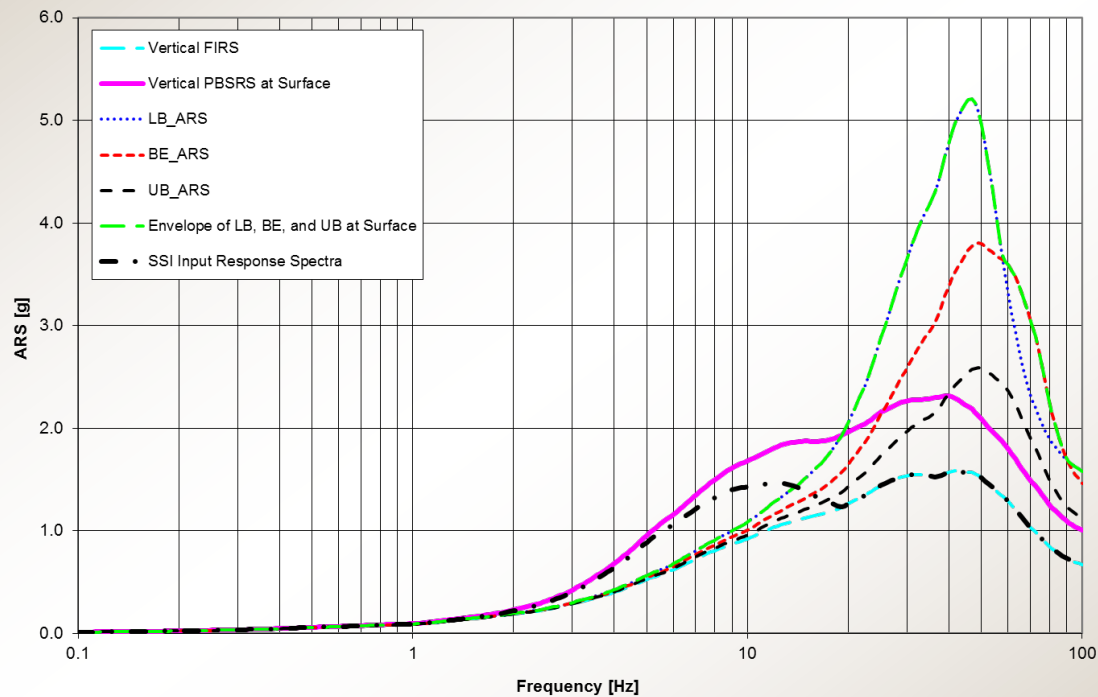
Comparison of V/H Ratio

# Anomalies Associated with P-wave Site Amplification



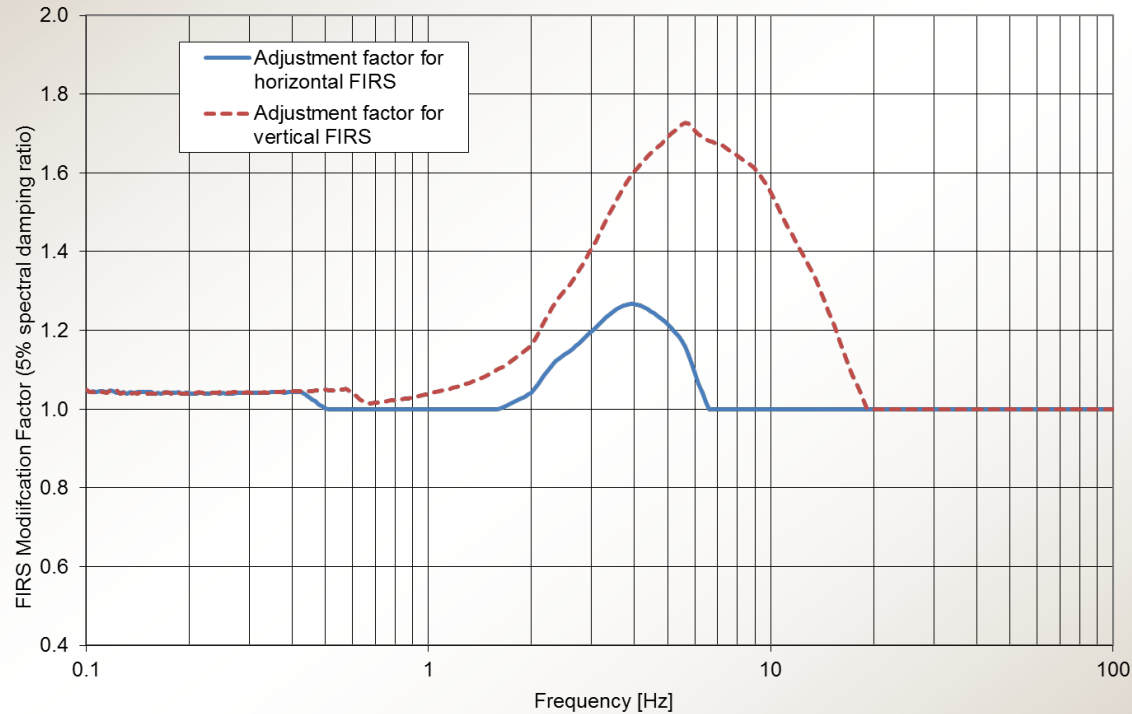
Shear and P-wave Velocity Profiles at a Rock Plant Site in US

# Anomalies Associated with P-wave Site Amplification



Vertical Responses at the Surface

# Anomalies Associated with P-wave Site Amplification



Adjustment Factors to be Applied to FIRS to Obtain SSI  
Input Response Spectra



# Anomalies Associated with P-wave Site Amplification

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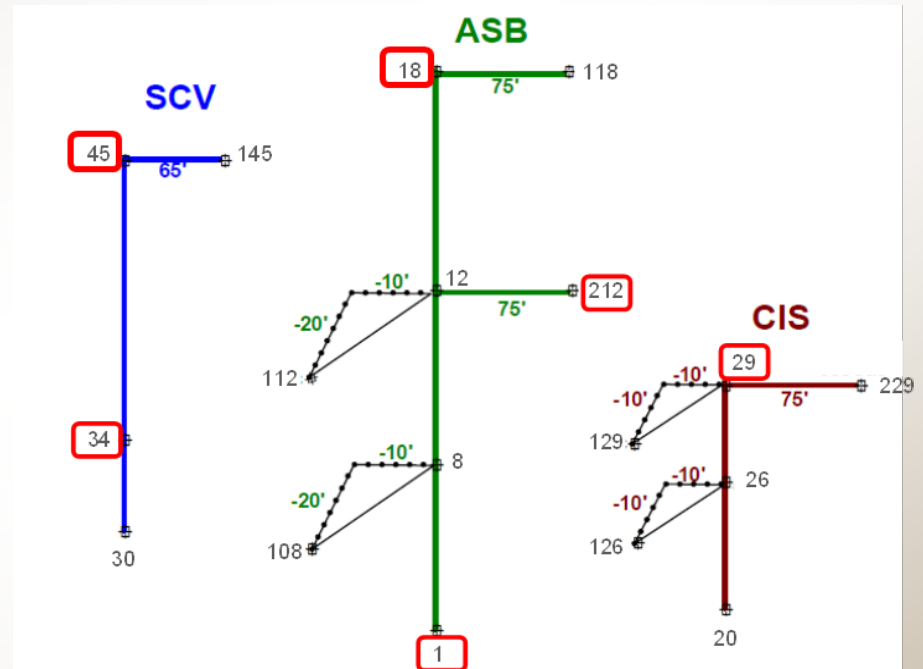
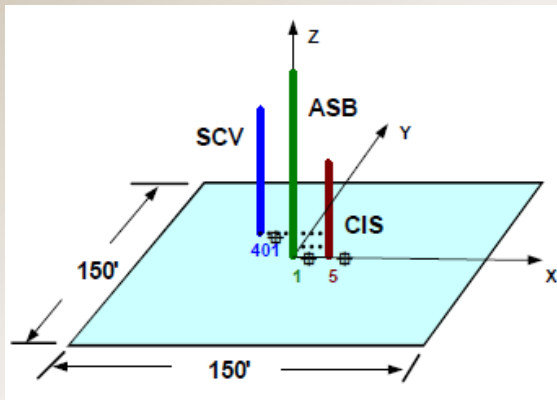
- There are anomalies and over prediction of vertical ground motion when P-wave propagation is used
- Several studies concluded that the vertical motion at low and mid frequency is the results of refraction and reflection of shear waves and not from P-wave body waves
- The P-wave analysis results conflict empirical V/H ratio relationship developed based on recorded motion
- The V/H ratio operators operate on H spectra to get the V spectra

# UNR Shear Box

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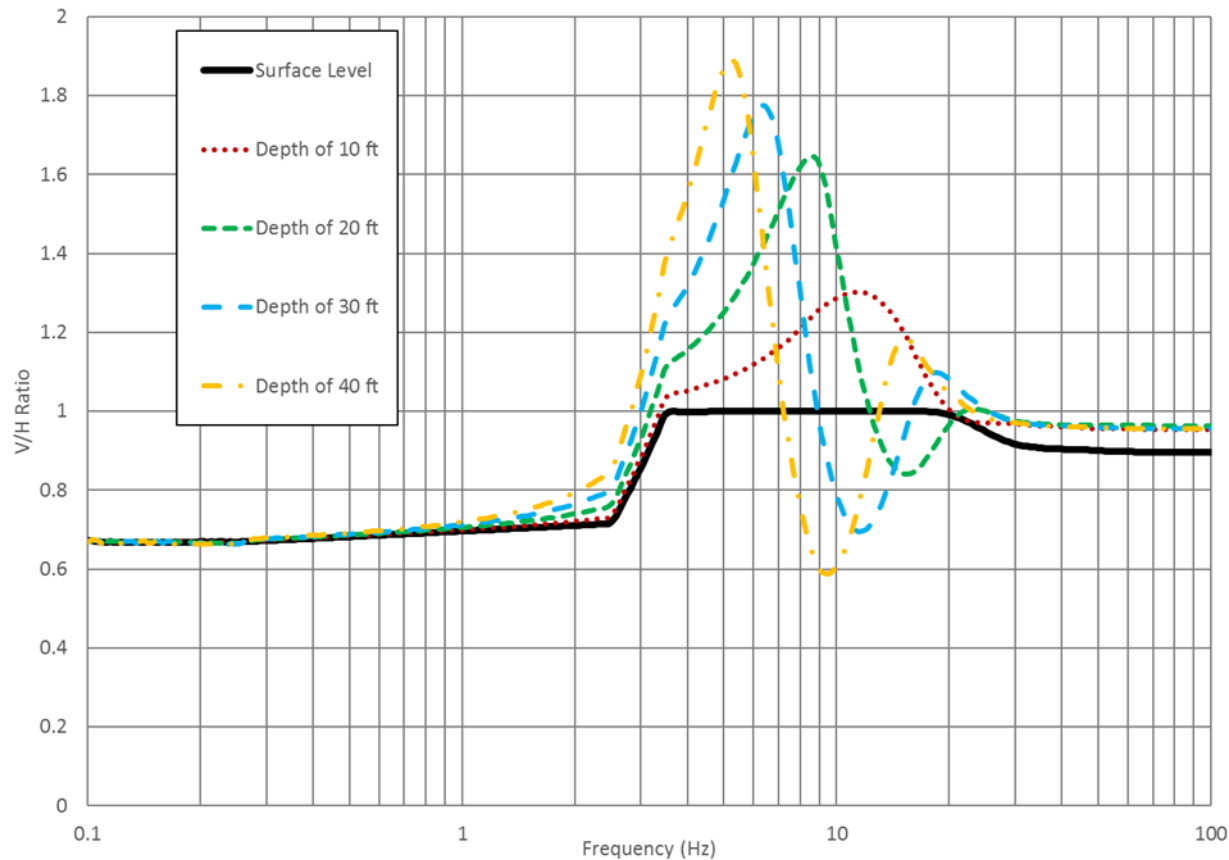
- In development of the horizontal spectra, the UNR shear box offer unique opportunities to evaluate:
  - ✓ Effect of one-dimensional versus two-dimensional shaking on soil nonlinearity and site response
  - ✓ Assessment and verification of equivalent linear soil models versus nonlinear models and their limits
  - ✓ Validation data for site response nonlinear analysis
  - ✓ Site properties at high level of soil strain particularly soil damping (out of reach of laboratory testing, RCTS)
  - ✓ Resonance of thin soft soil layers on rock or stiff soil layers
  - ✓ Begin to provide SSI data for extreme shaking for validation of nonlinear SSI solutions

# SSI Case Study



Adopted AP1000 Lumped Mass Stick Model (LMSM)

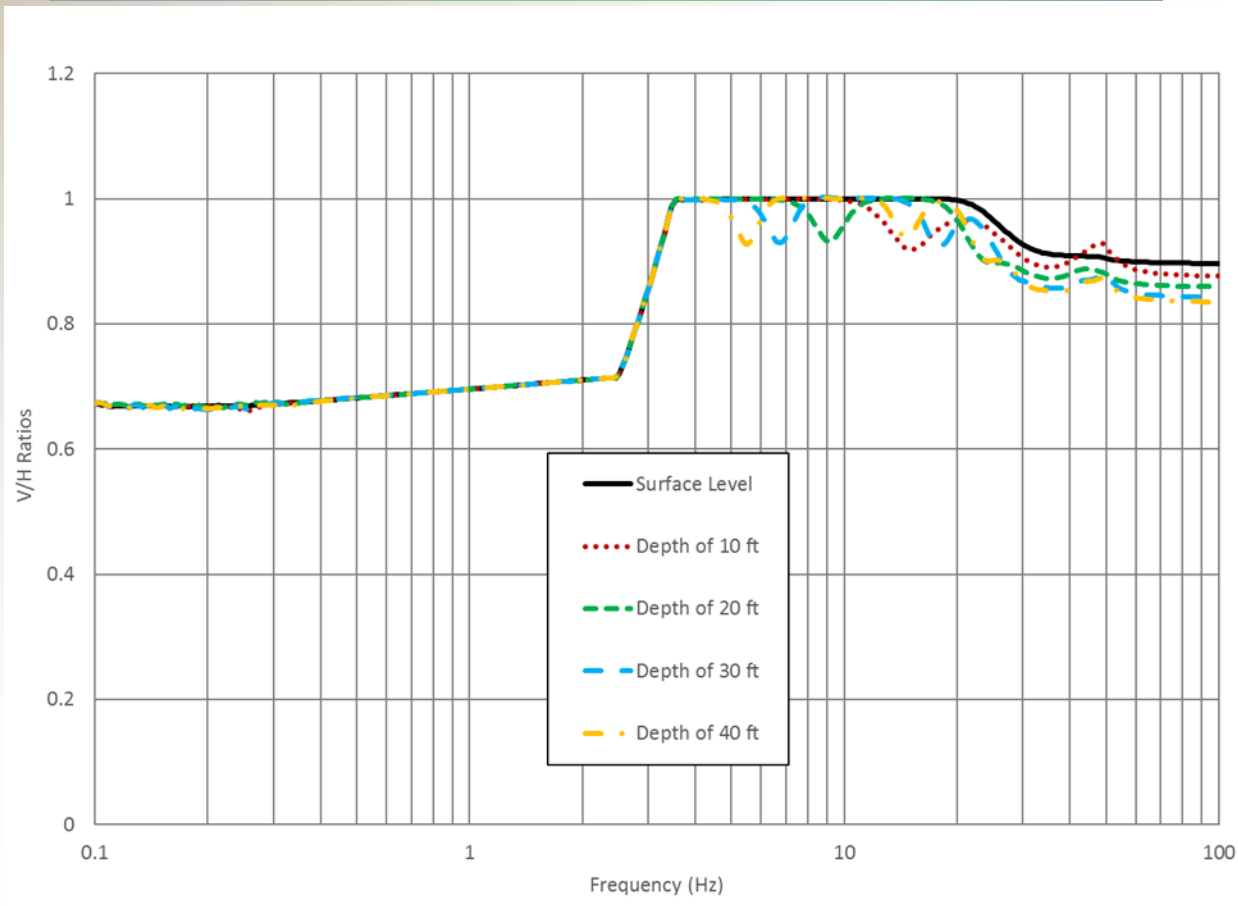
# SSI Case Study



V/H ratios in the Soil Profile due to P-wave Propagation Input, RG 1.60 Input Motion



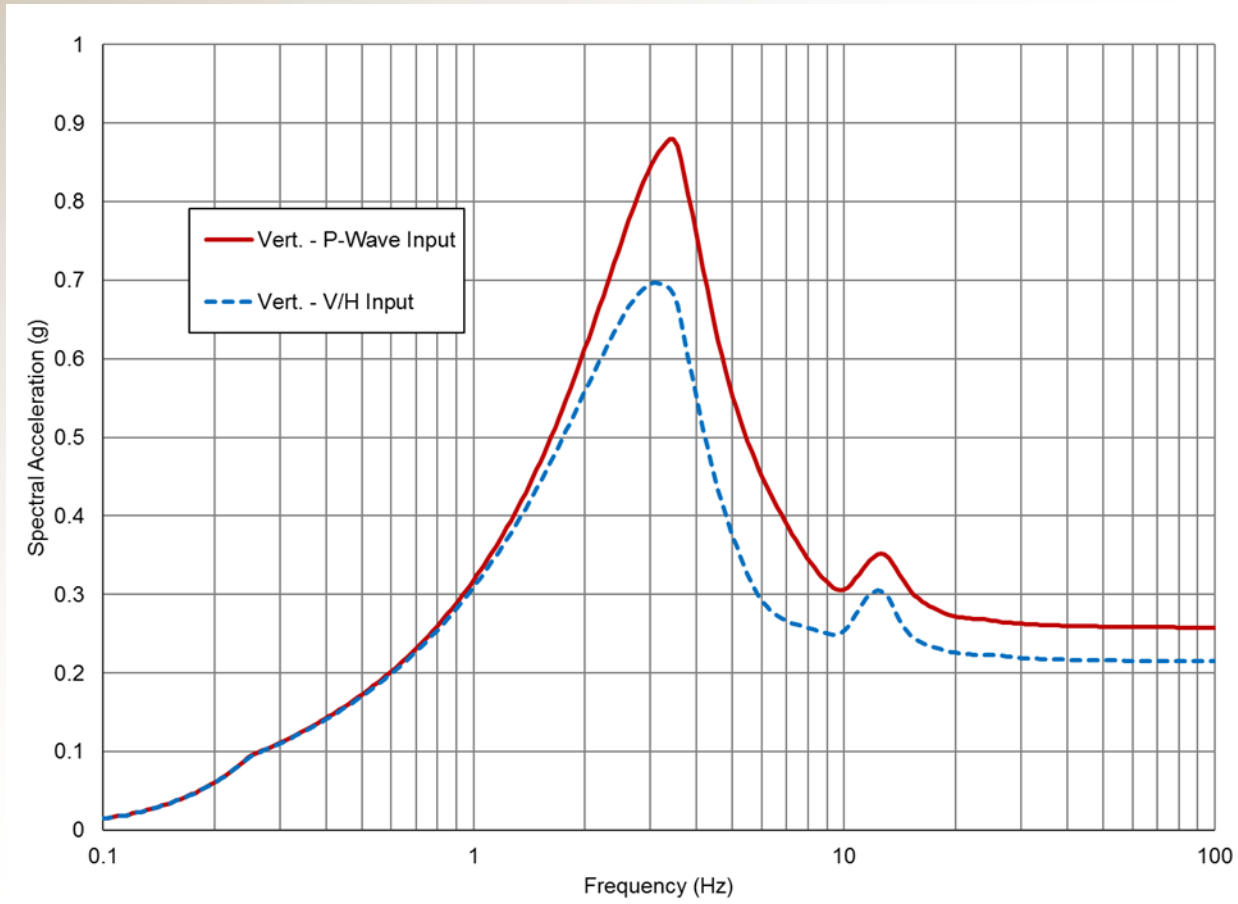
# SSI Case Study



V/H ratios in the Soil Profile due to Consistent V/H Input, RG 1.60 Input Motion



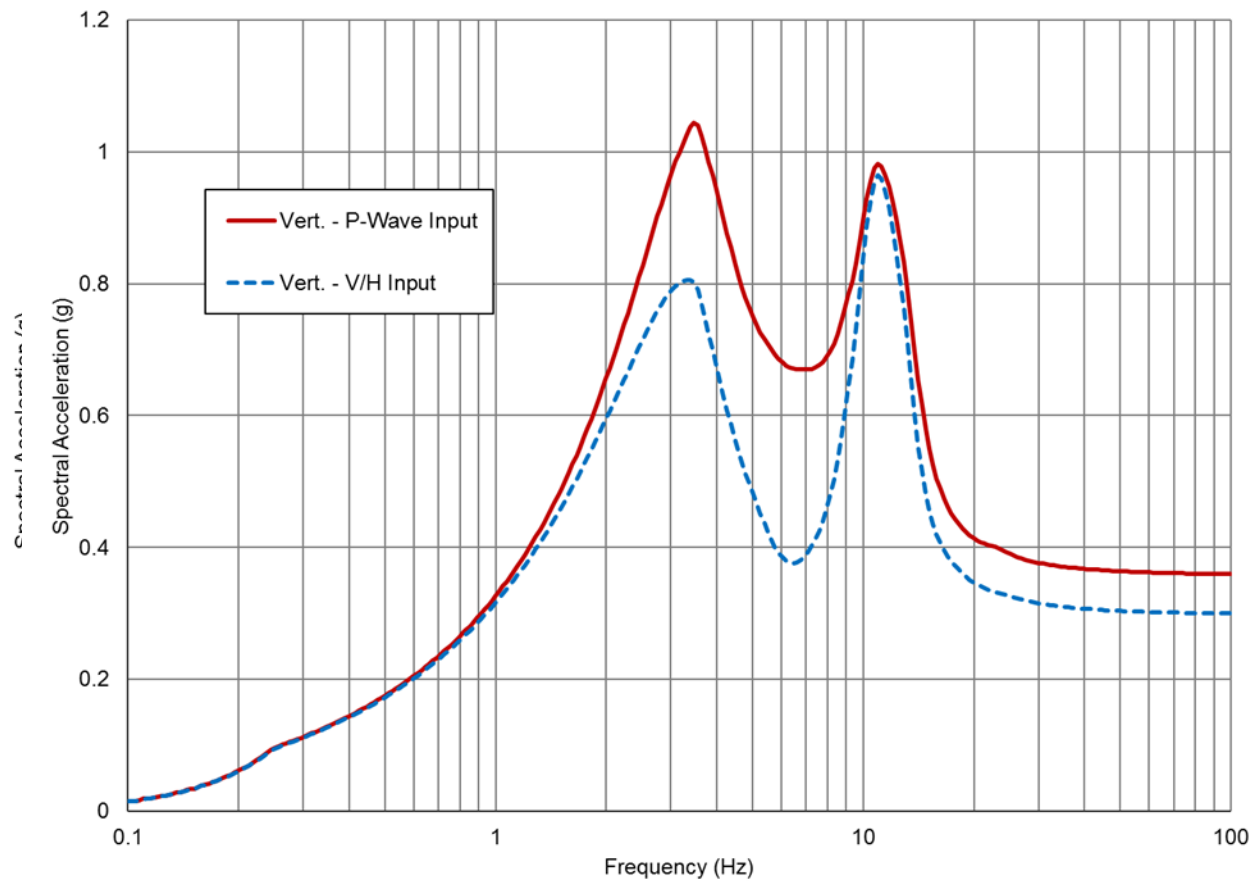
# SSI Case Study



Comparison of Vertical ARS at Node 1 on Foundation

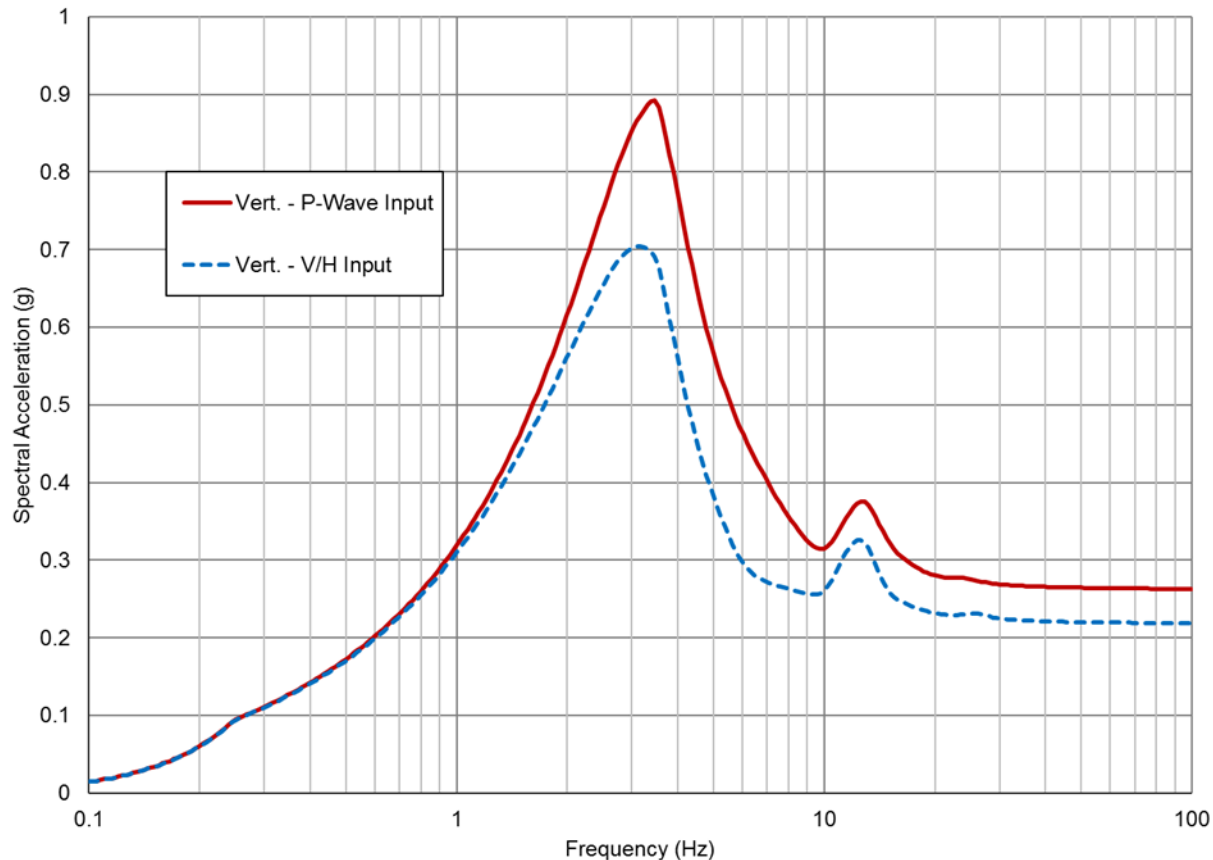


# SSI Case Study



Comparison of Vertical ARS at Node 18, top of ASB

# SSI Case Study



Comparison of Vertical ARS at Node 29, Top of CIS

# SSI Case Study

Total Vertical Seismic Load (kips)			
	ASB	CIS	SCV
P-Wave Input	$2.608 \times 10^4$	$2.600 \times 10^4$	$2.403 \times 10^3$
Consistent V/H Input	$2.137 \times 10^4$	$2.130 \times 10^4$	$1.925 \times 10^3$
Mean Basemat Pressure (ksf)			
	ASB	CIS	SCV
P-Wave Input	1.159	1.156	0.107
Consistent V/H Input	0.950	0.947	0.086

Comparison AP1000 on Deep Soil Profile: Total Vertical Seismic Load and Mean Basemat Seismic Pressure



# Closure

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Consistent V/H Ratio Approach for vertical SSI analysis has been approved by ASCE 4 committee for implementation in ASCE 4-22 in progress at this time



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Thank You  
Comments/Questions

