

Liquefaction of gravelly soils and the impact on critical infrastructure

Adda Athanasopoulos-Zekkos, PhD

Assistant Professor

University of California, Berkeley

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Liquefaction of Gravels and their Impact on Infrastructure

- 1948 Fukui EQ – gravelly sand
- 1964 Alaska EQ – sandy gravel
- 1975 Haicheng EQ – gravelly sand
- 1976 Tangshan EQ – gravel and sand
- 1976 Friuli, Italy EQ – gravel and sand
- 1983 Borah Peak EQ – silt and gravel
- 1993 Hokkaido-Nansei-Oki EQ – gravelly sand
- 1995 Kobe EQ – sandy gravel
- 1999 Chi-Chi EQ – gravel, sand, silt

2014 Cephalonia EQ



Nikolaou et al. GEER (2014)

2016 Kaikoura EQ

- 2008 Wenchuan EQ – gravel and sand
- 2014 Cephalonia EQ – gravel
- 2016 Kaikoura EQ – gravelly sand



Cubrinovski et al. (2018)



M_w 6.1
02/03/2014



Atokos

January 26 and February 3 2014 Cephalonia, Greece EQ, $M_w = 6.1$

4 Major Ports:

- **Devastating** Lixouri
- **Moderate** Argostoli
- **Minor/Insignificant** Sami
- **No Damage** Poros

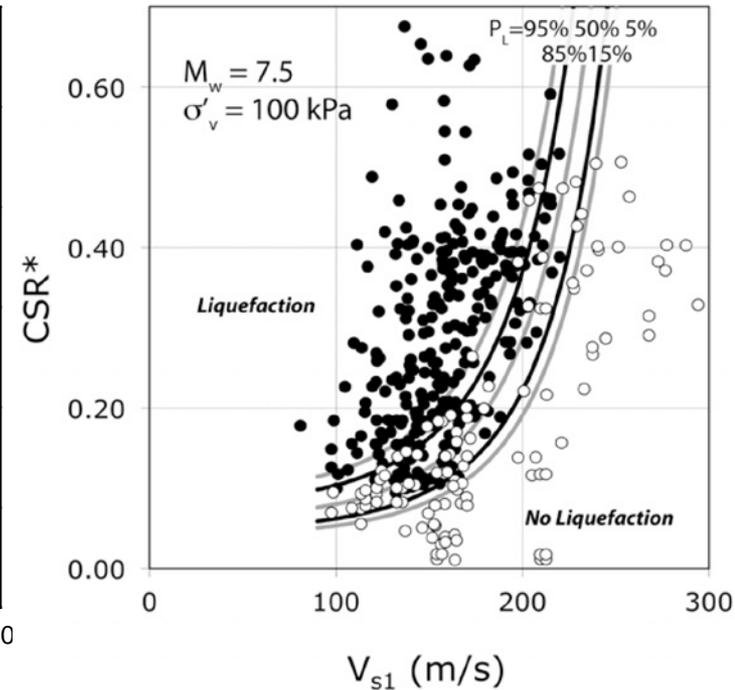
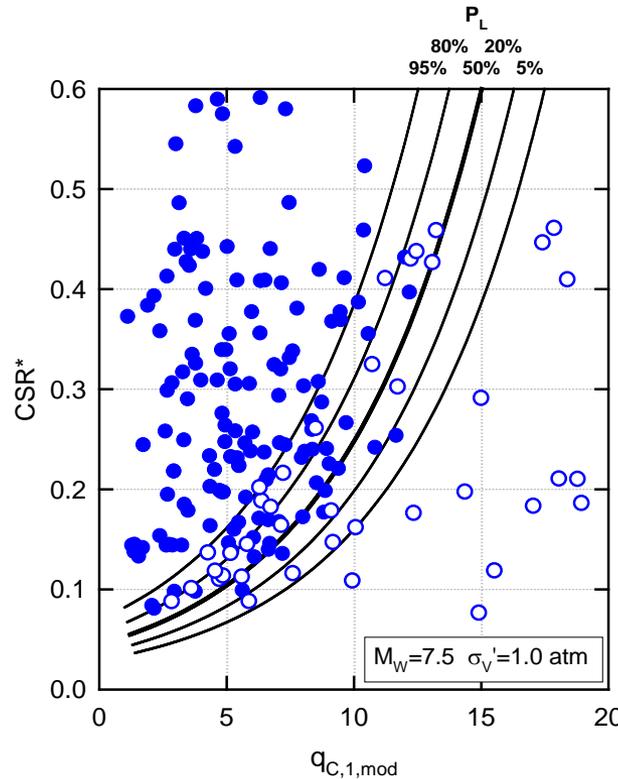
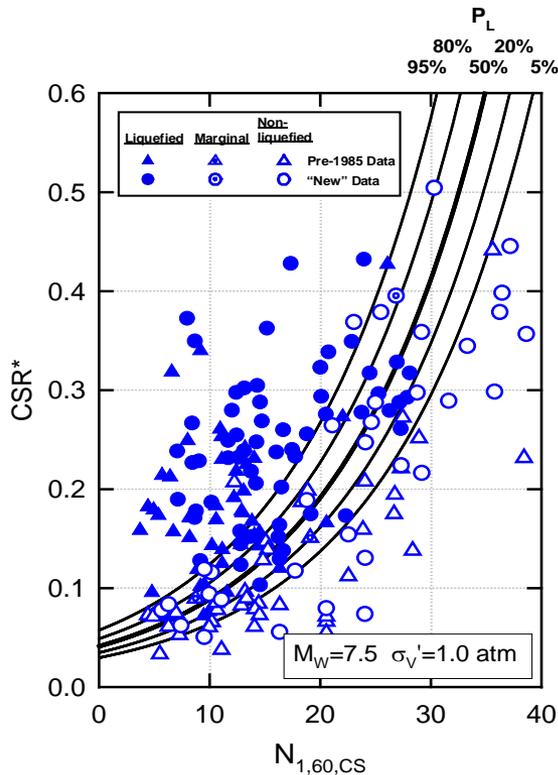


Reliable assessment charts have been developed for sands, but not for gravels

SPT

CPT

Vs



(Cetin et al, 2004; Moss et al, 2006; Kayen et al, 2013)

Gravels are challenging to characterize in the field due to their particle size



Becker Penetration Test
(BPT)



Dynamic Penetration Test
(DPT)



Shear Wave Velocity
Measurements (V_s)

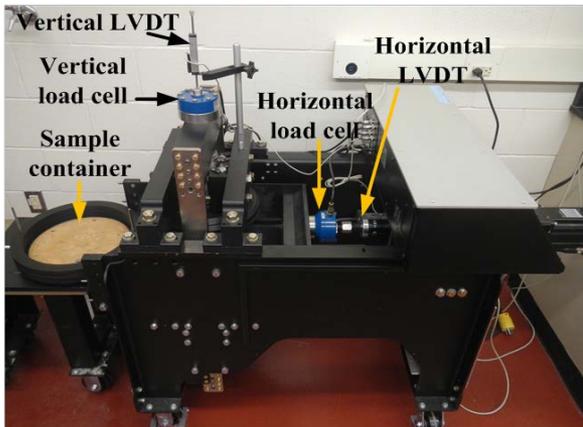
Gravelly soils can also be difficult to test in the laboratory

- Need large devices to accurately capture response
- Significant time to prepare specimens
- Most tests are Triaxial - Possible membrane compliance issues (Evans and Seed, 1987; Nicholson, Seed and Anwar, 1989)



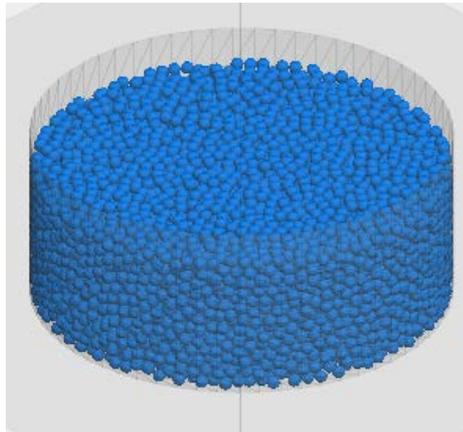
Integrated approach: Micro to Macro Response

Laboratory Testing



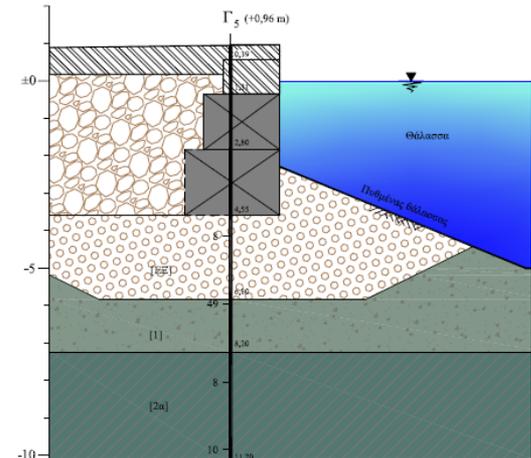
Large-scale CSS used for constant-volume monotonic, cyclic, and post-cyclic shear tests with V_s measurements

Numerical Modeling



3D DEM analyses

Field Response



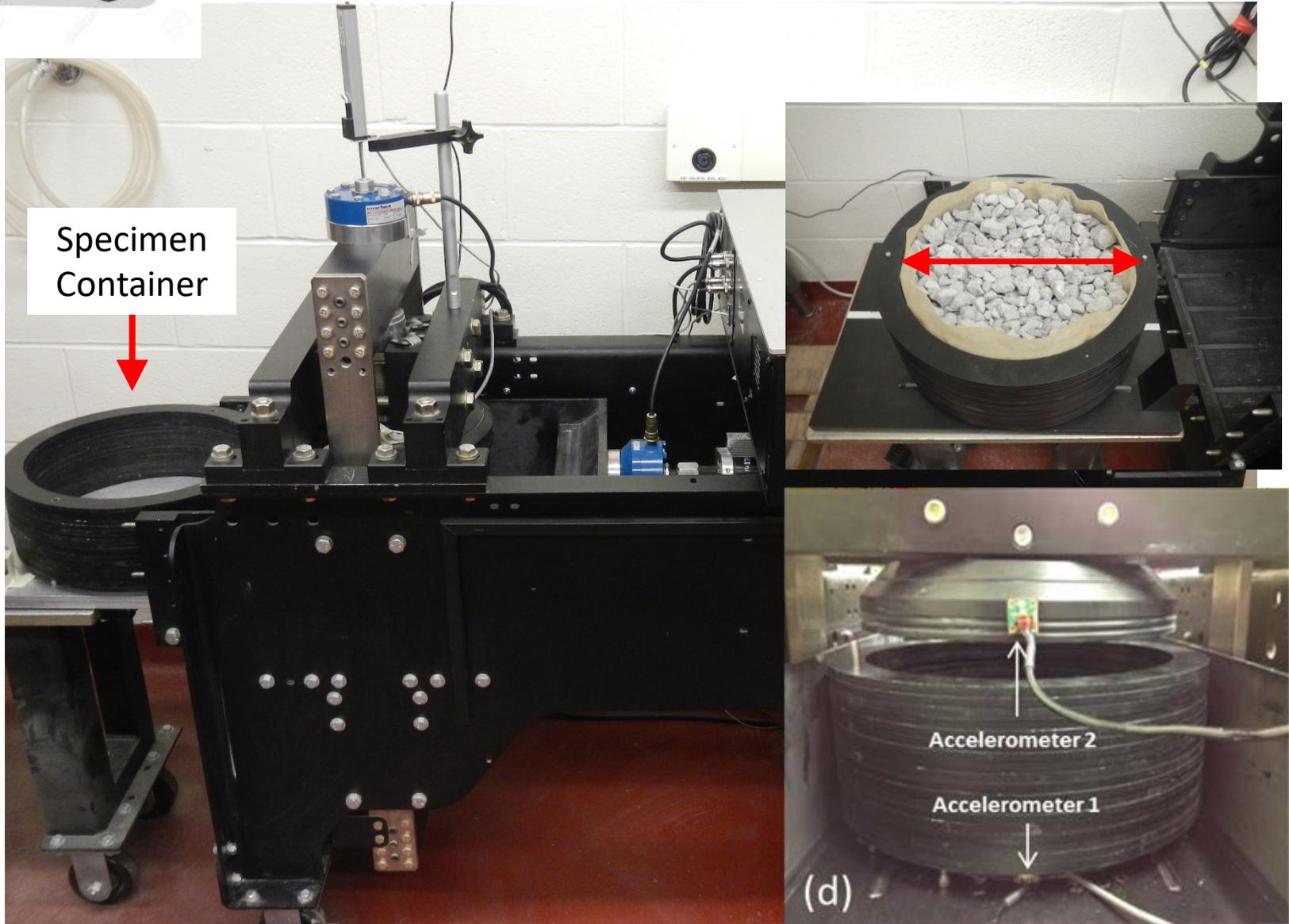
V_s and DPT measurements in the field.

Back-analysis of case histories.



Cal

Large-size Cyclic Simple Shear (CSS)



Monotonic, cyclic and post-cyclic tests were performed on three uniform gravels,

Pea Gravel



Rounded to
Subrounded

8 mm Crushed
Limestone



Angular

5 mm Crushed
Limestone

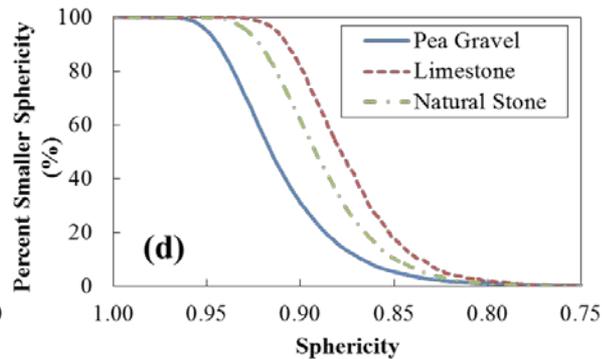
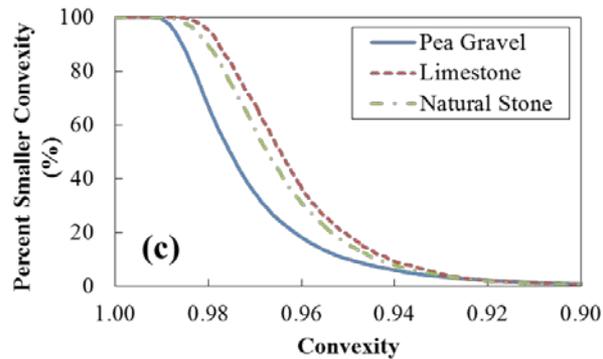
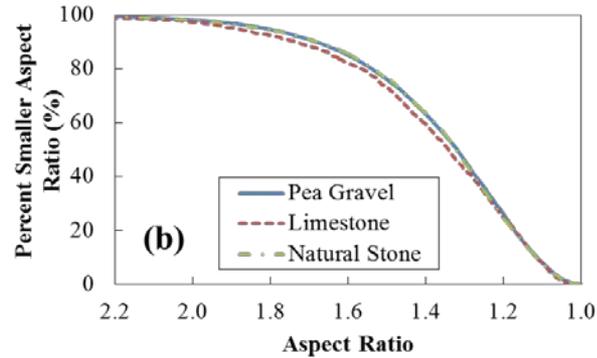
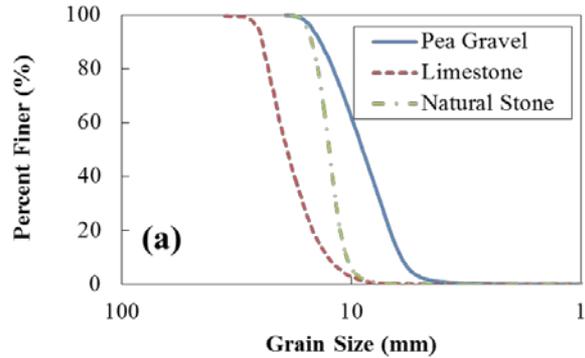


Angular

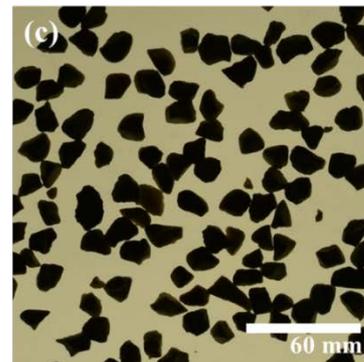
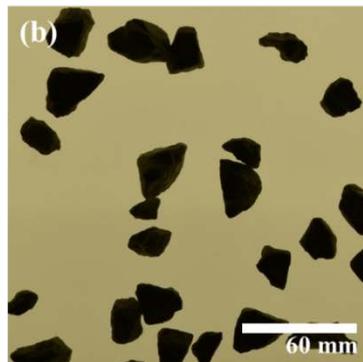
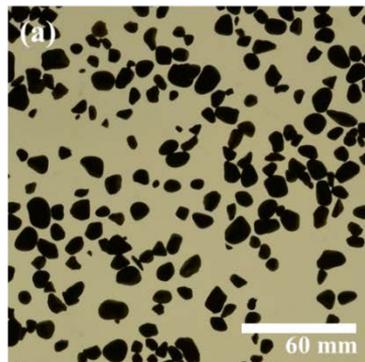
V_s was measured in every specimen



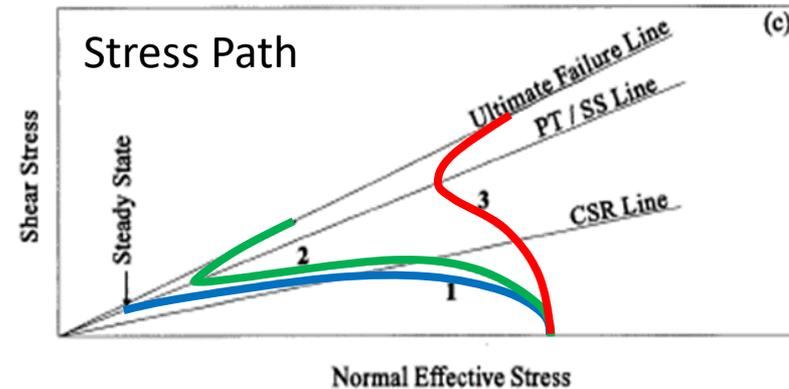
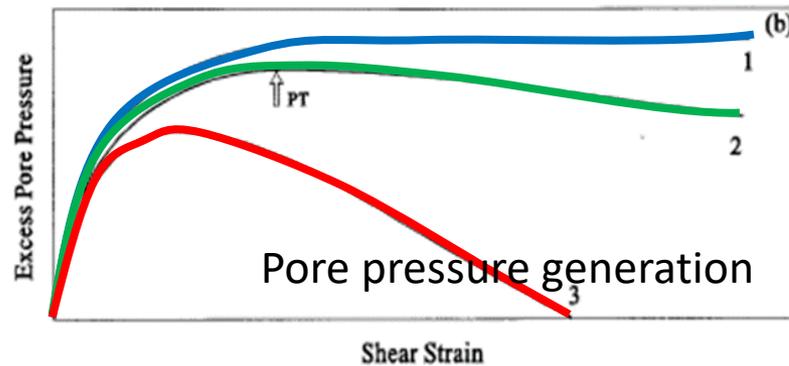
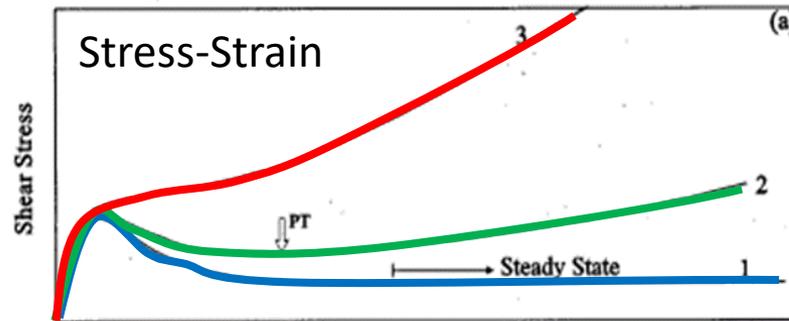
Translucent Segregation Test for Particle Morphology



Test per Ohm and Hryciw, 2013

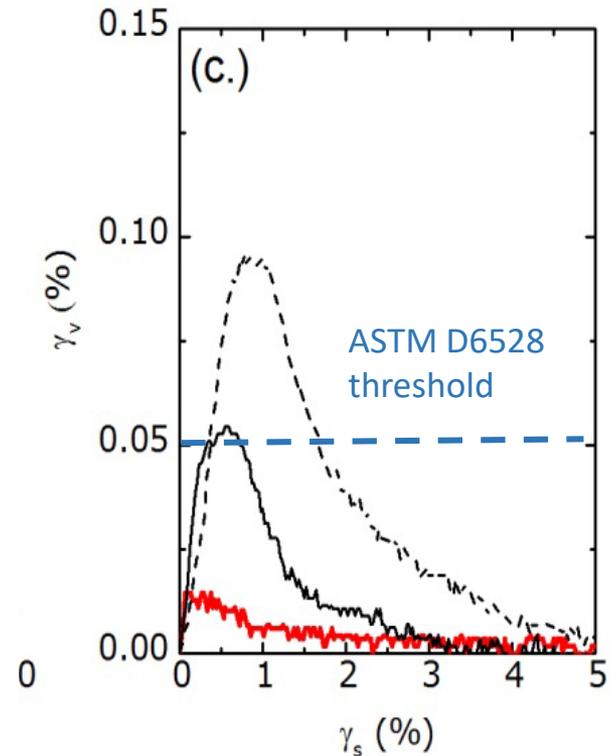
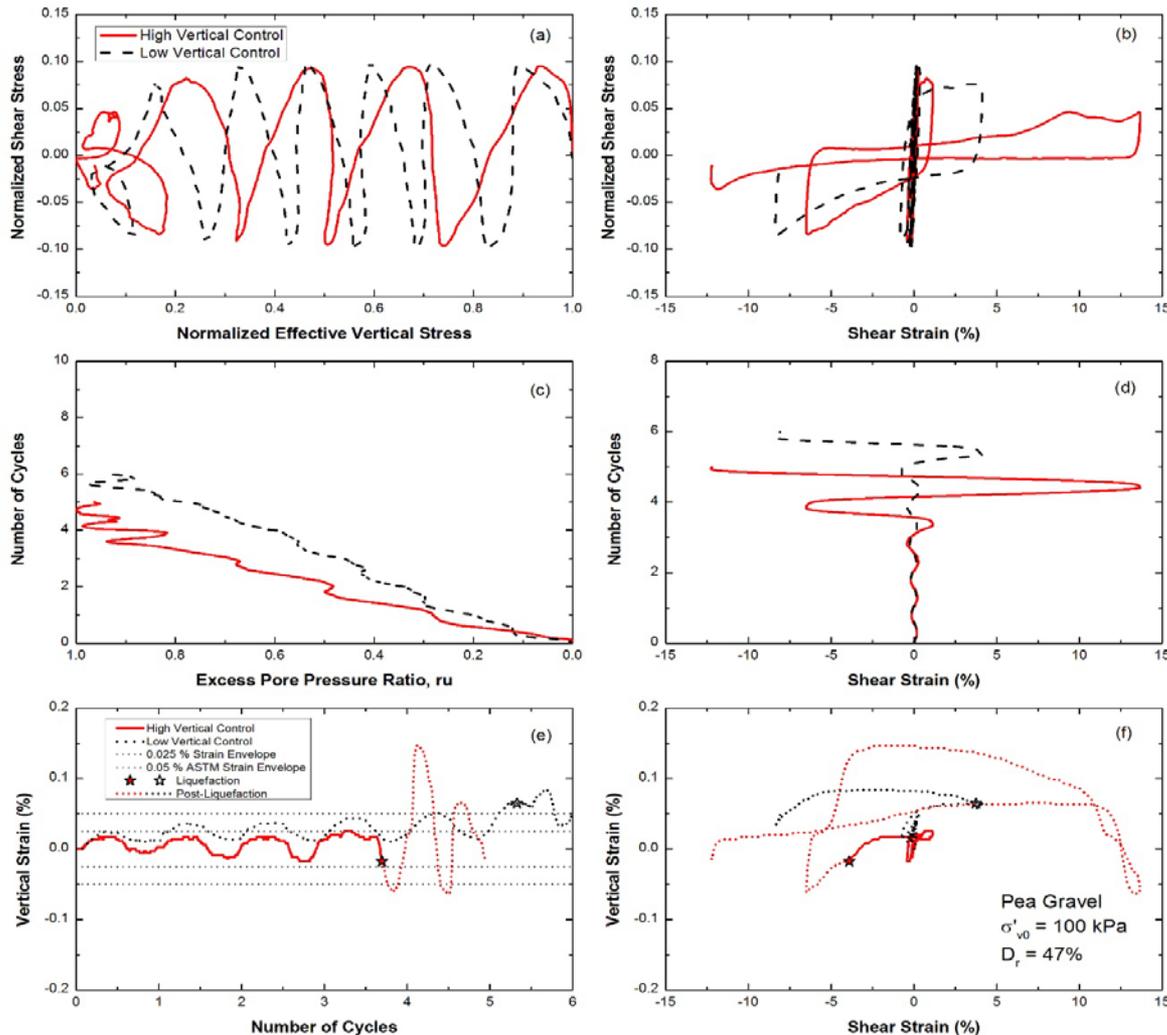


Critical state-based framework for granular soils



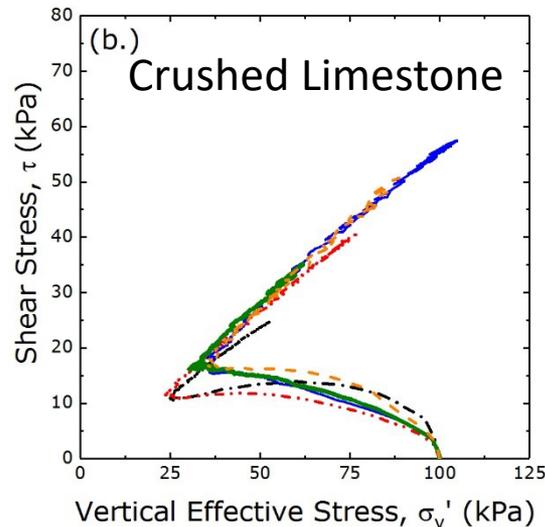
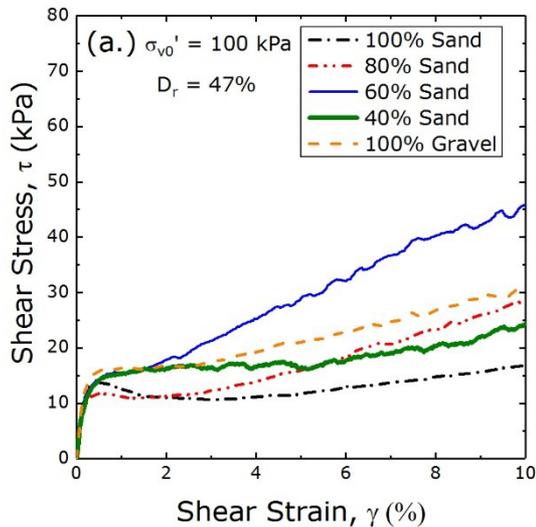
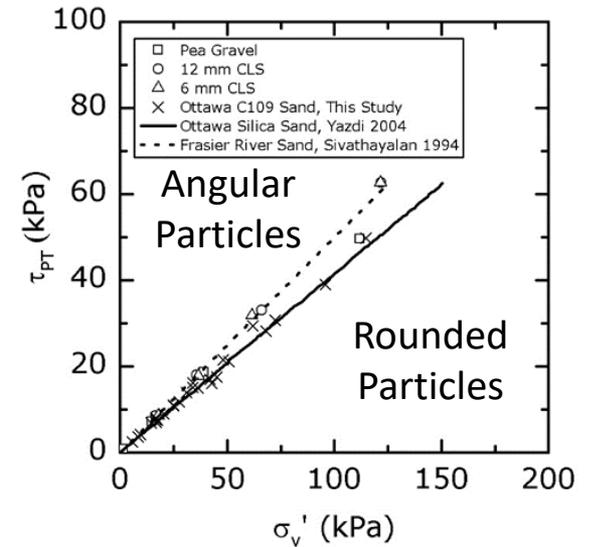
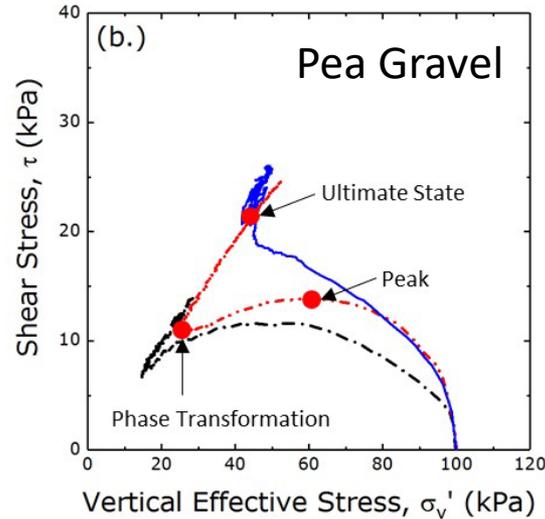
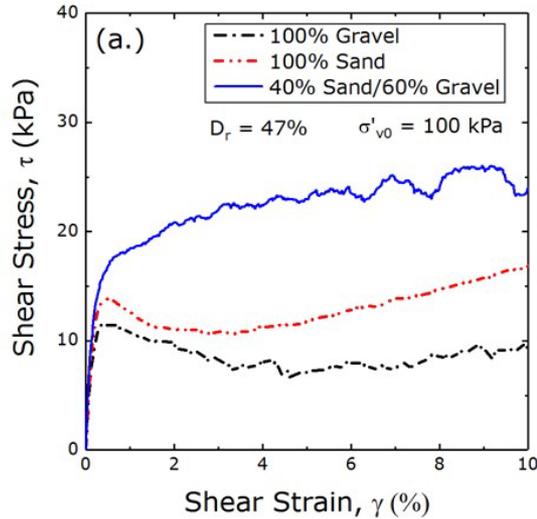
We expect gravels to follow a similar framework

True constant volume conditions?

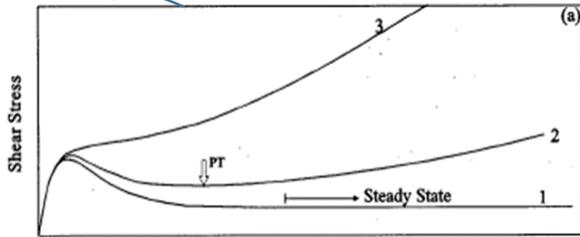


ASTM D6528 threshold needs to be reduced by 50%, and evolution of vertical strain should be always reported.

Effect of particle angularity is important



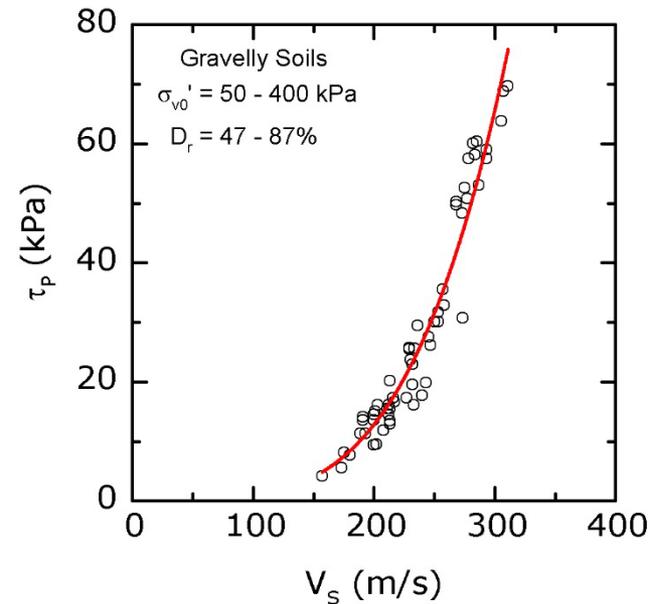
Correlation of Shear Wave Velocity with peak, phase transformation and ultimate state



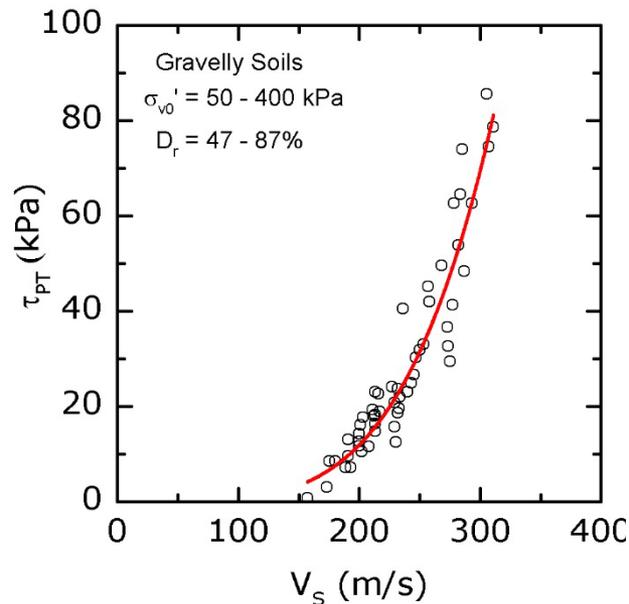
Peak

Phase
Transformation

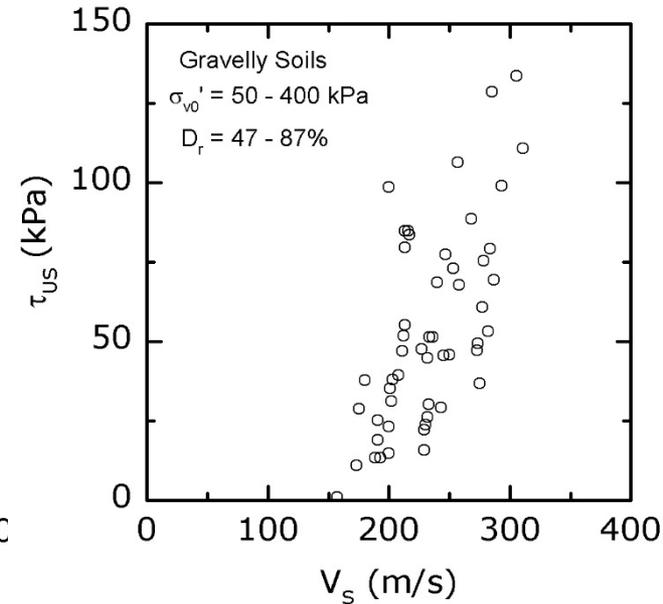
Ultimate State



Shear Strain = 0-1%

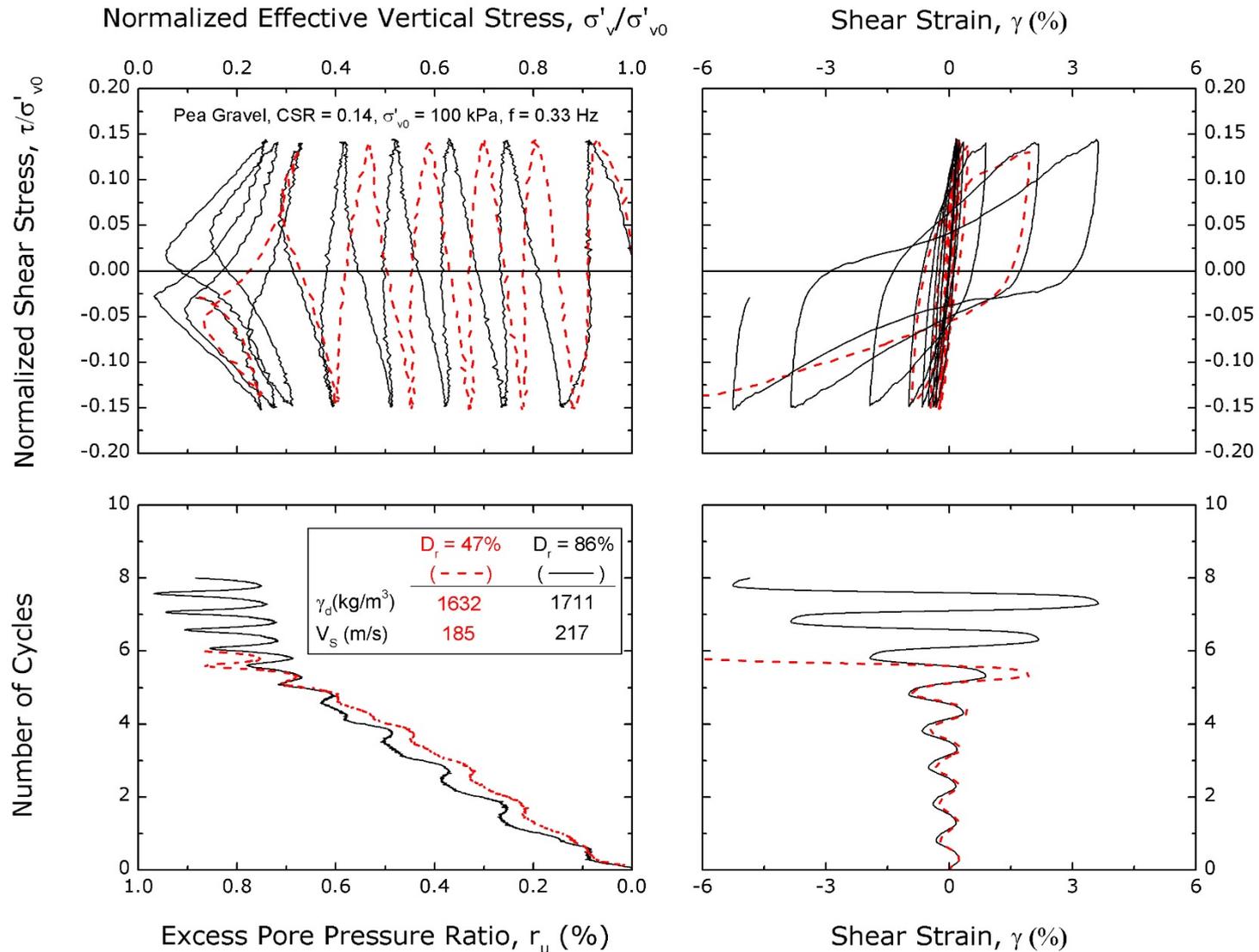


Shear Strain = 1-5%

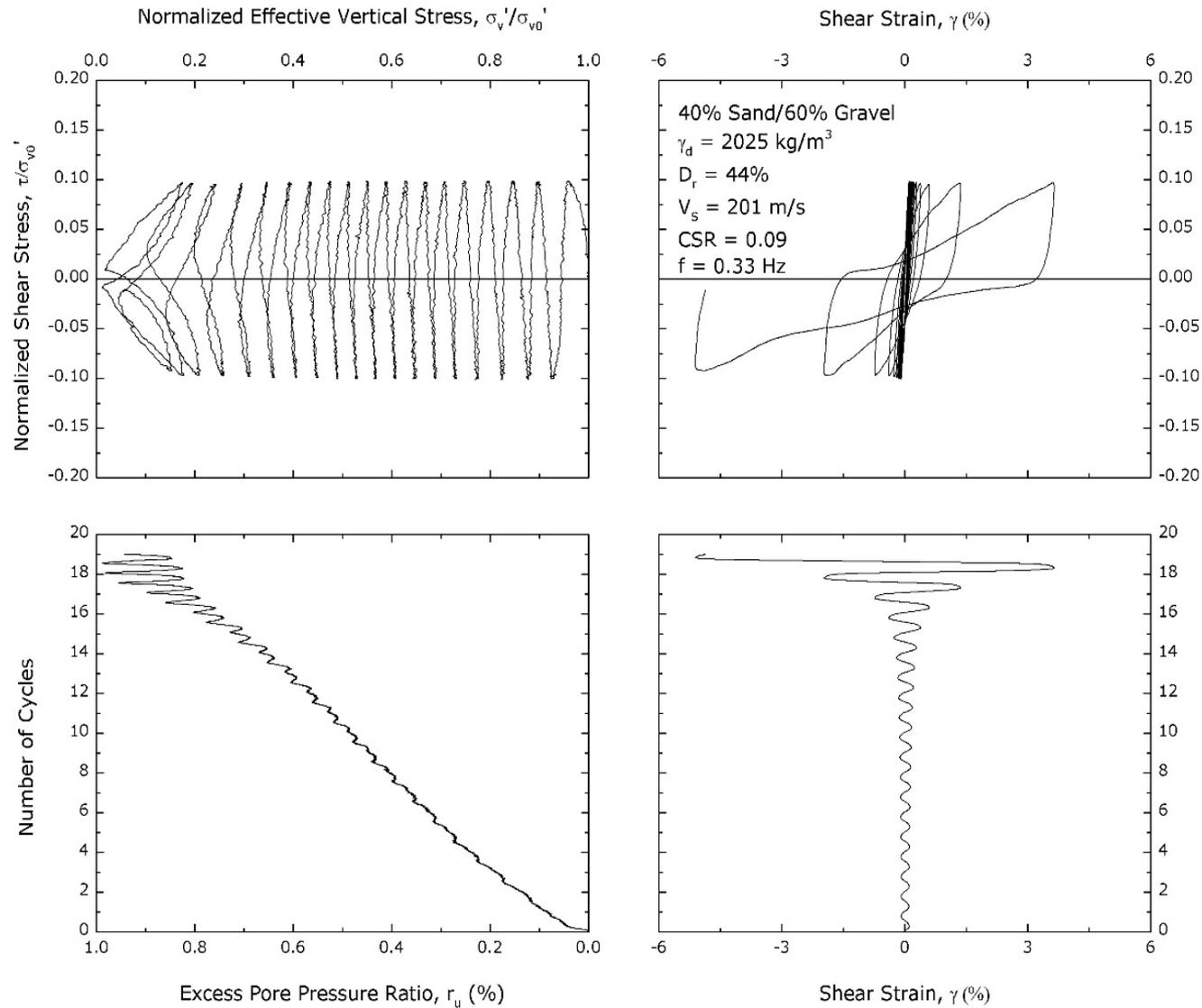


Shear Strain = 15-20%

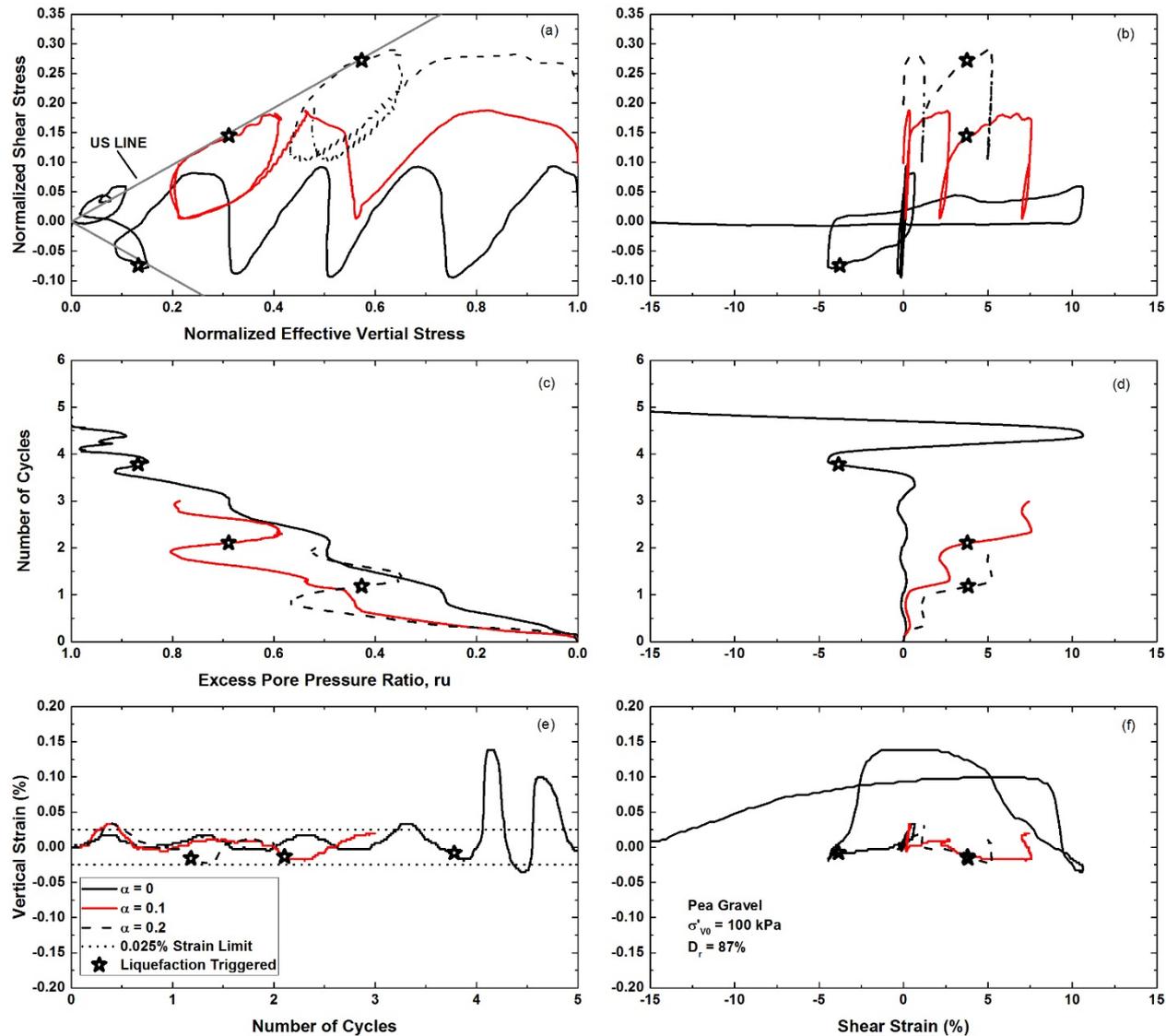
Cyclic Simple Shear Test Results for Pea Gravel



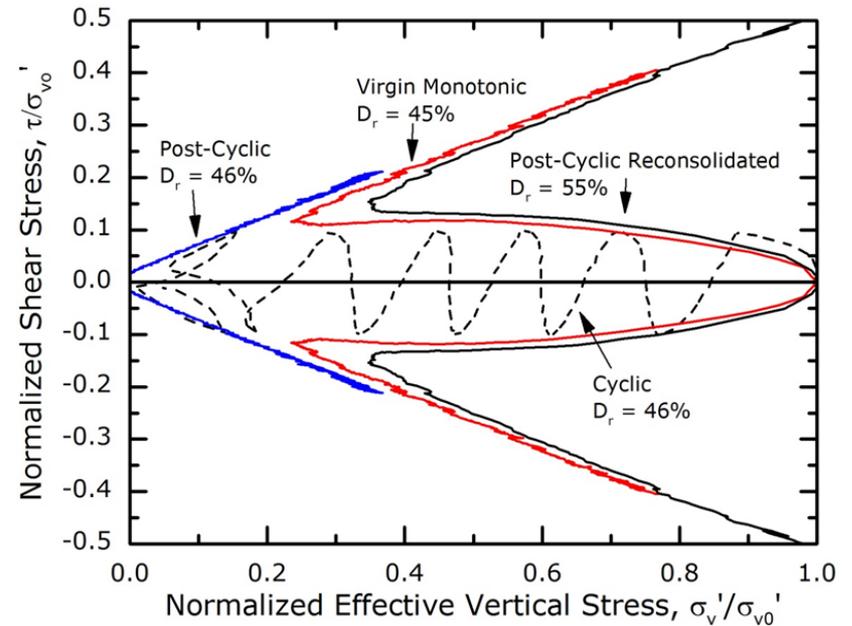
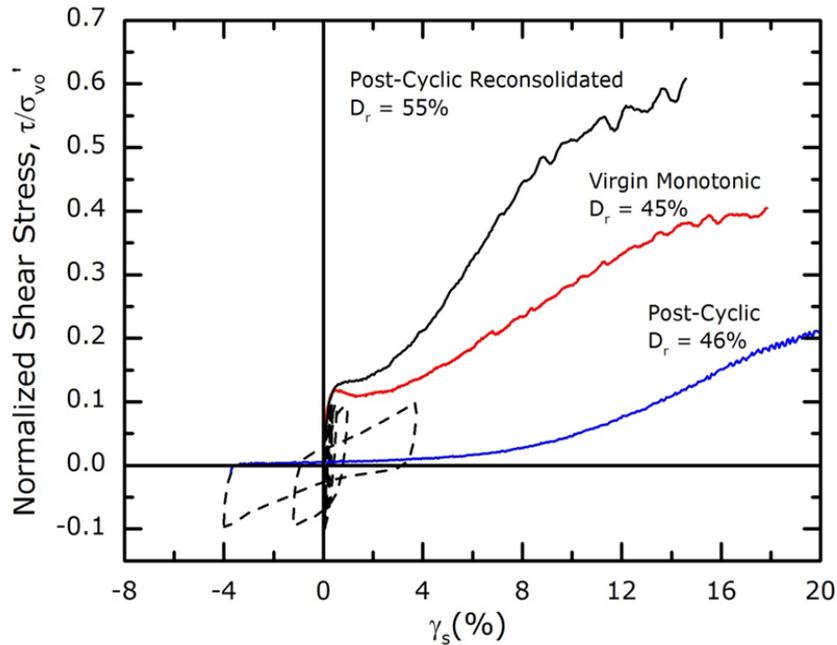
Cyclic Simple Shear Test Results for Sand/Gravel Mix



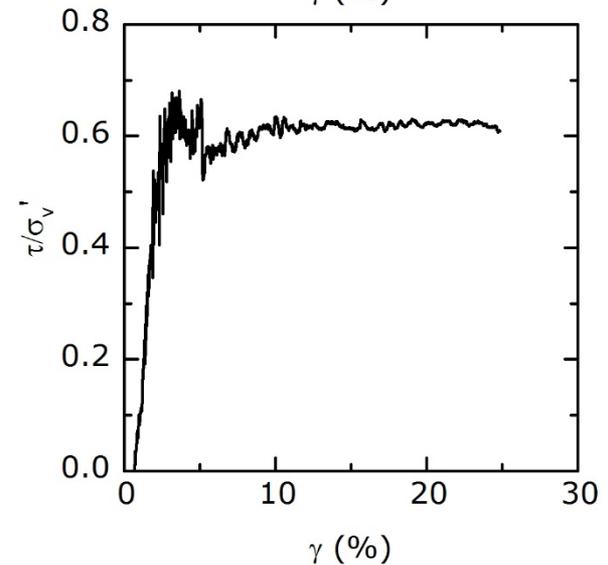
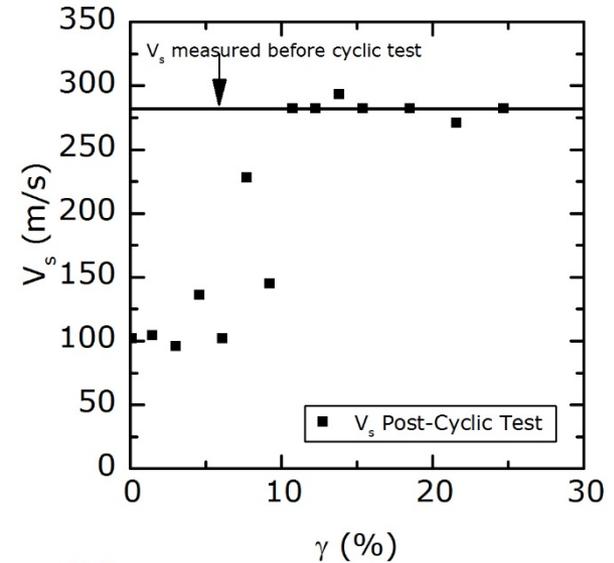
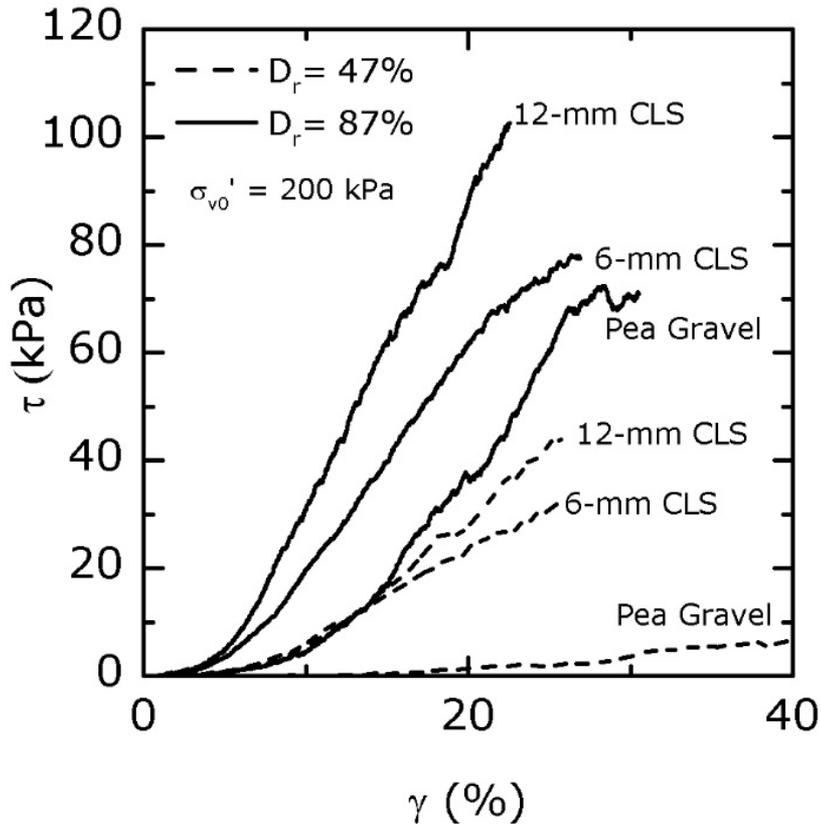
Cyclic Simple Shear Test Results for Gravel- $K\alpha$



Monotonic shear results give insight into cyclic and post-cyclic shear response

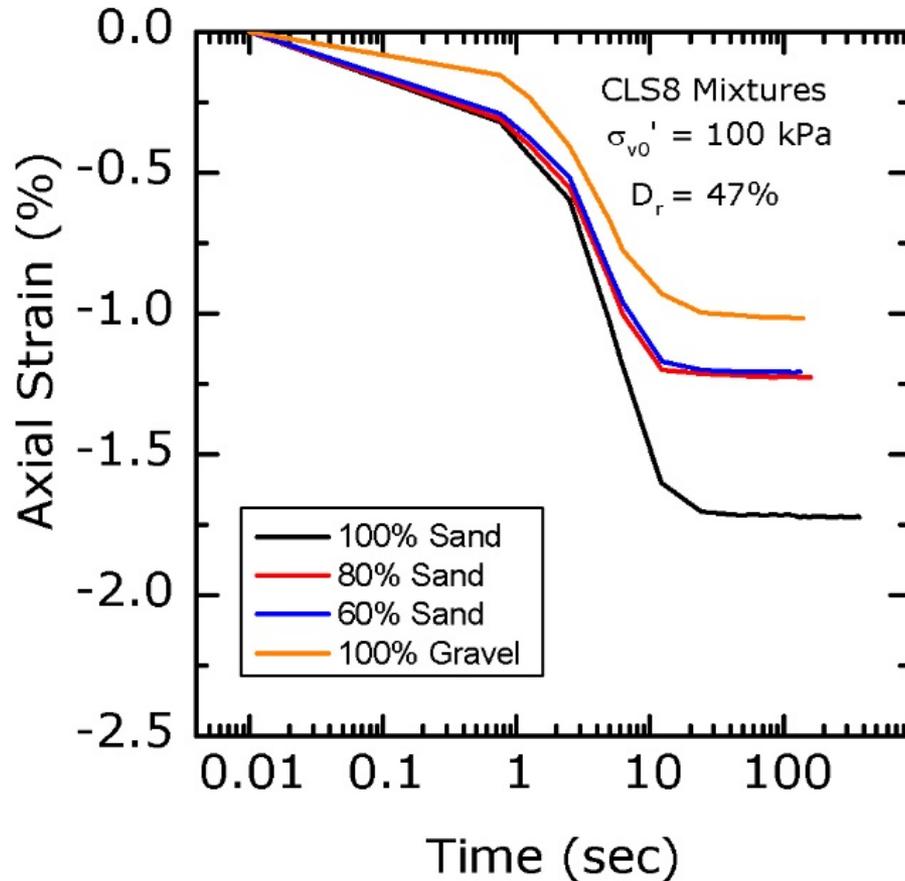


Post-cyclic shear response of Gravels



- Post-liquefaction shear resistance is affected by particle angularity

Post-cyclic shear response for gravelly soils



Typical Volumetric strains:

All Denser specimens: ~ 1%

Looser Pea Gravel 1.5%

Looser Crushed Limestone
1.0%

Looser Ottawa Sand ~2%

Field testing was performed in Utah, Alaska and Greece to compare with laboratory results

Ferron, UT



Cephalonia, Greece



Valdez, Alaska



DPT and Vs measurements were performed at both Cephalonia ports

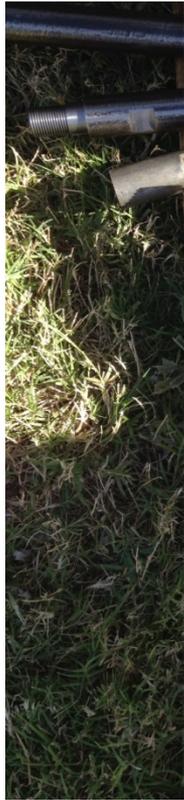


DPT Testing Rig



Athanasopoulos-Zekkos, A., Zekkos, D., Rollins, K., Hubler, J., Higbee, J. and Platis, A. (2019). "Earthquake Performance and Characterization of Gravel-Size Earthfills in the Ports of Cephalonia, Greece, following the 2014 Earthquakes", 7th International Conference on Earthquake Geotechnical Engineering, Rome 17-20 June, 2019

DPT Cone and Instrumented Rod for Energy Measurements

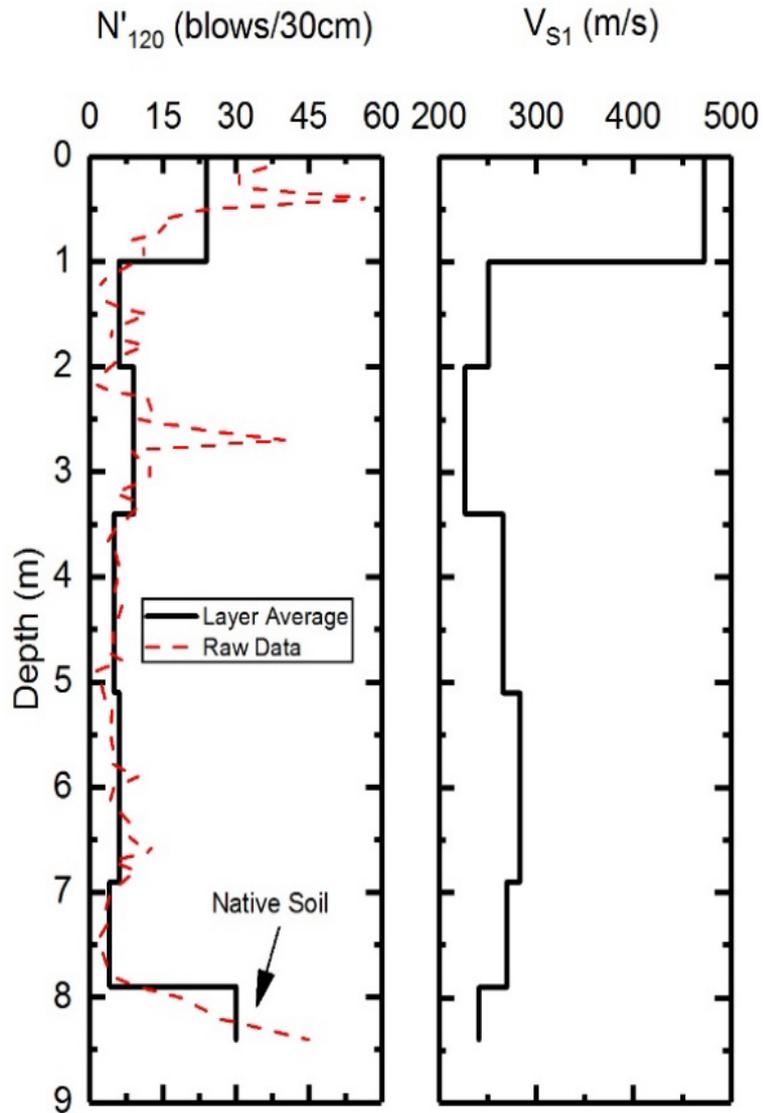


V_s Measurement setup – MASW

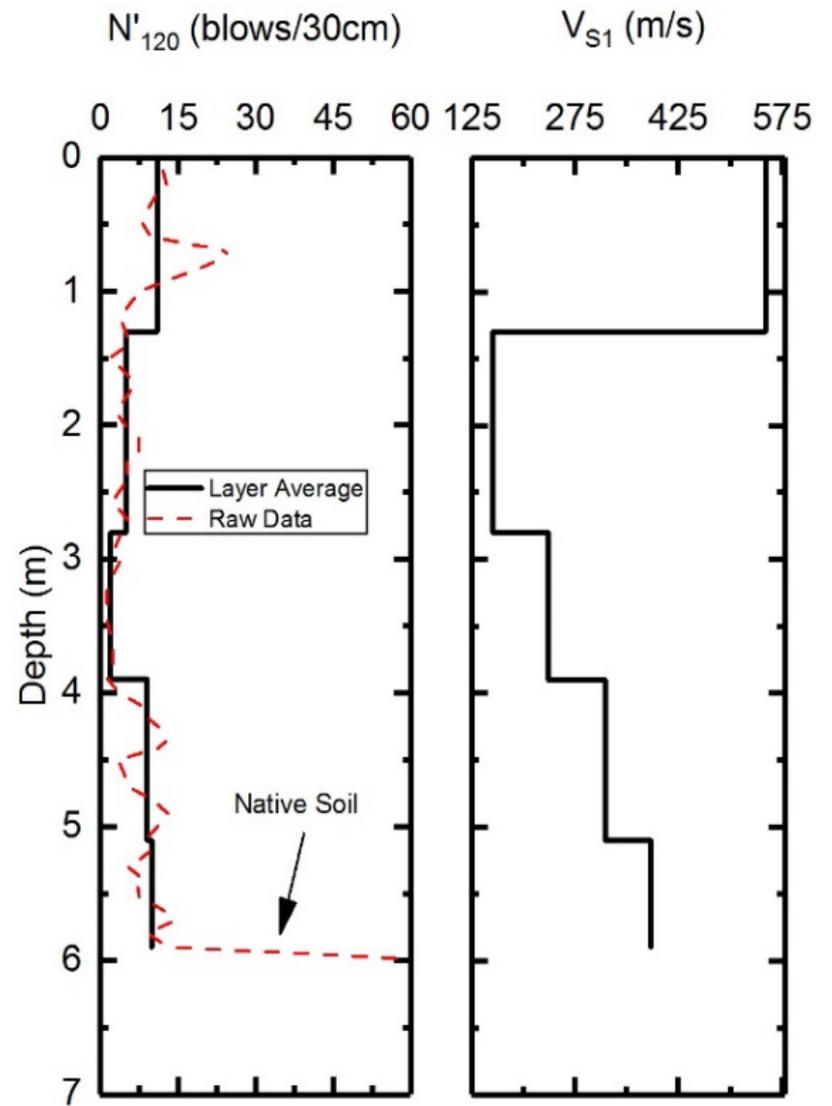


DPT and V_s correlate well at test locations

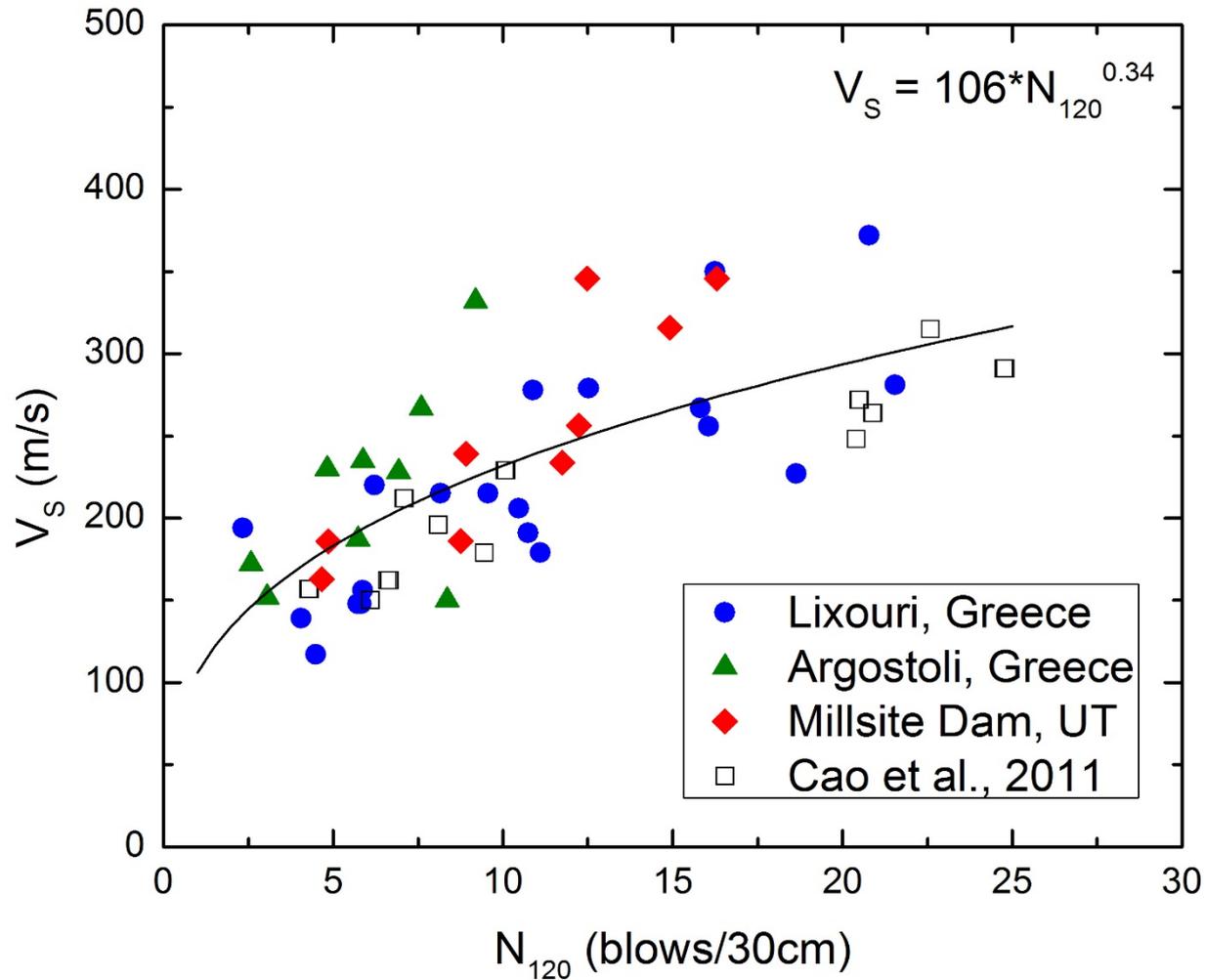
Argostoli Port



Lixouri Port

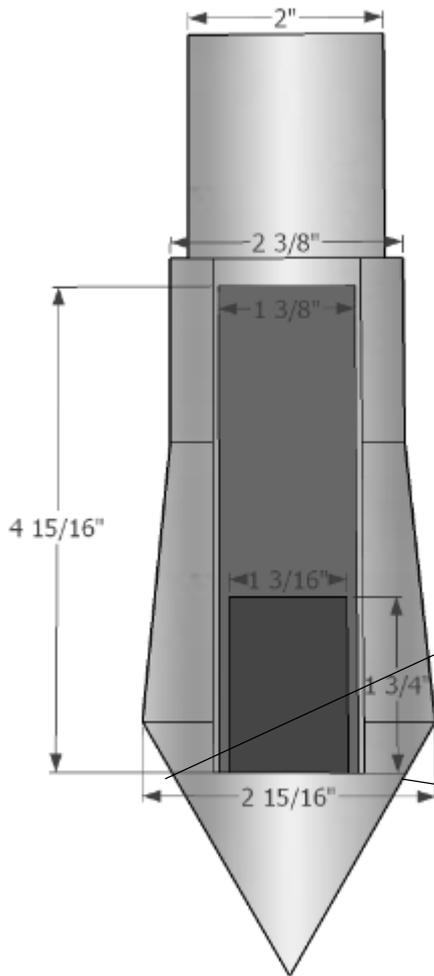


Correlation between DPT and V_s



eDPT: Adding new sensing capabilities

TYPICAL DPT CONE



PDA equipment:

Accelerometer:

20,000 g range, 4.5 kHz Freq range

Size: 45 x 25 x 30 mm

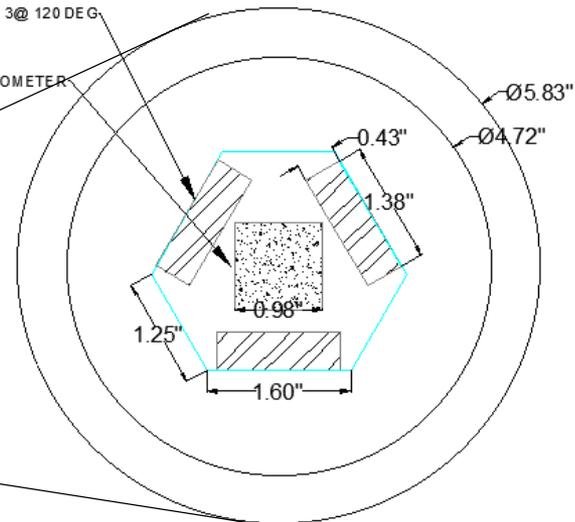
Strain gage:

3000 micro-strain range

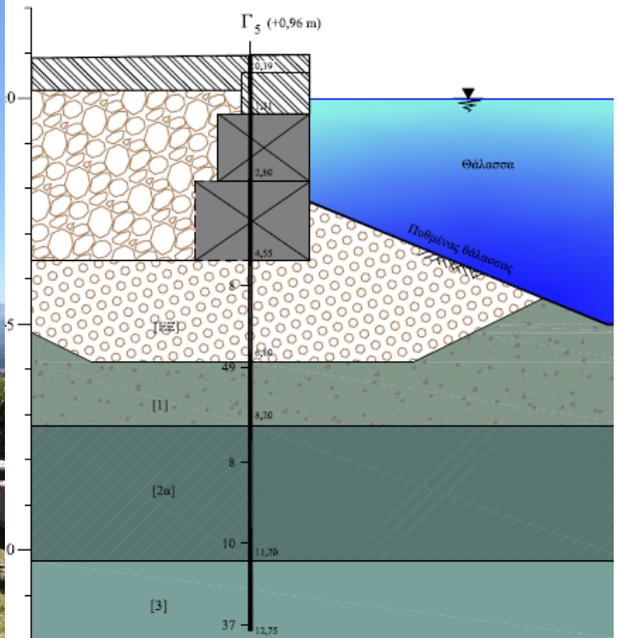
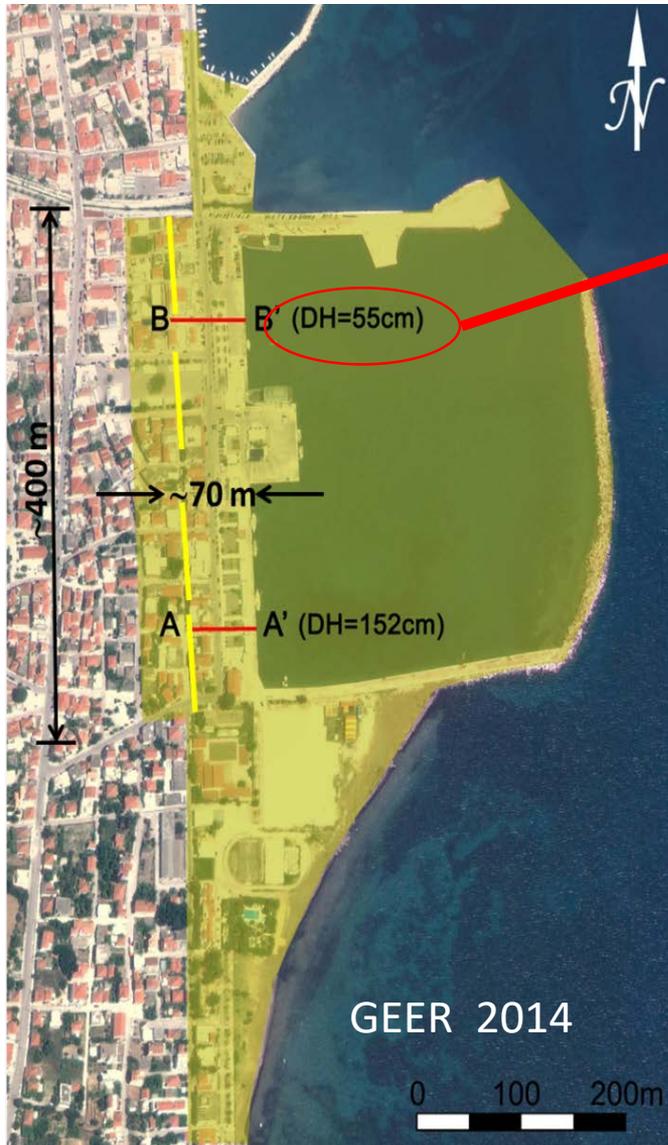
Size: 126 x 35 x 11 mm

STRAIN GAGE 3@ 120 DEG

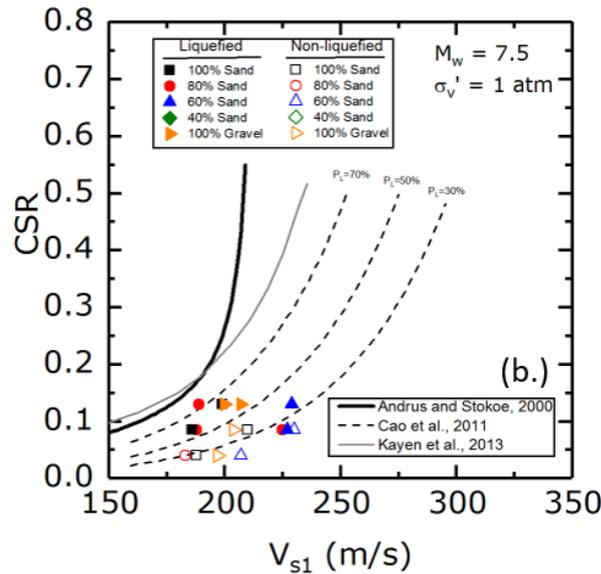
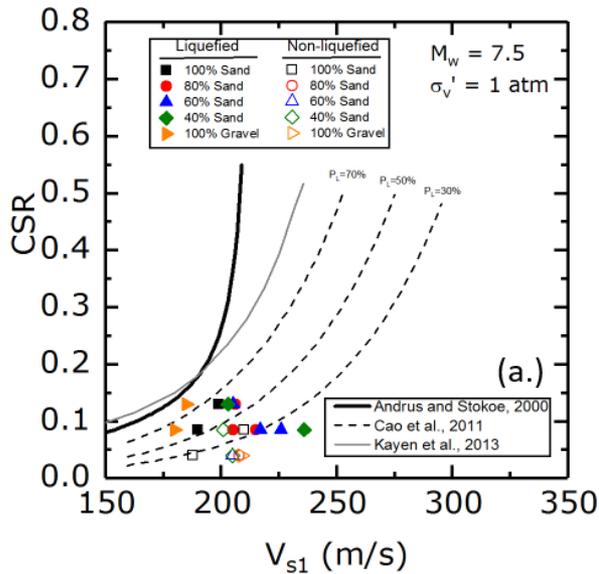
ACCELEROMETER



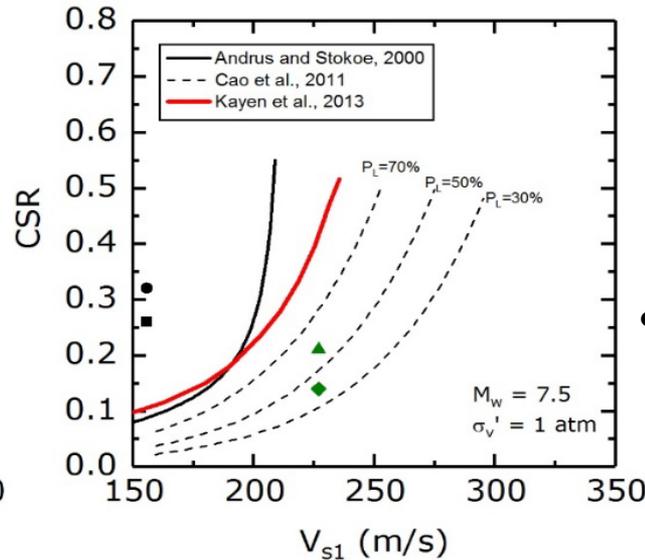
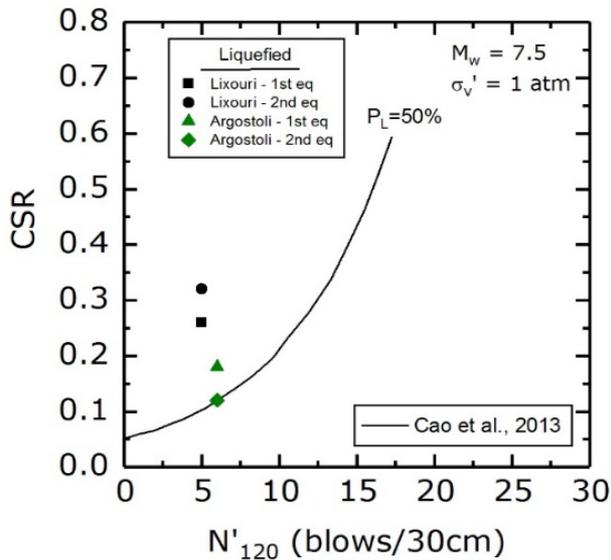
Field Testing and Numerical Modelling of Field Behavior



Comparison of laboratory tests and case history analysis results to existing relationships



- Gravels and Gravel Sand mixes are readily liquefiable in the **laboratory** even for $V_s > 200 \text{ m/s}$
- Gravelly soils liquefied **in the field** at higher V_s values than previously expected ($V_s > 200 \text{ m/s}$)



- Evidence of liquefaction in the field may not always be as pronounced due to layering, smaller volumetric strains
- Need to connect micro (e.g. DEM) to macro scale response (e.g. infrastructure)

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adda.zekkos@berkeley.edu

