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Stochastic Simulator-based Uncertainty Quantification for Seismic Responses of Bridges

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- **Title**: Stochastic simulator-based uncertainty quantification for seismic responses of bridges
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Research goals

- The goal is to develop an efficient stochastic simulator-based approach for seismic UQ analysis of bridge responses
- Technical aims are:
 - > Develop stochastic surrogate models for the stochastic simulator
 - > Perform UQ analysis for seismic responses using stochastic surrogate models
 - > Analyze **sensitivity indices** leveraging stochastic surrogate models

Research overview



1. Development of stochastic surrogate model

Challenges in surrogate modeling



• **Dimensionality reduction** can be useful

Dimensionality reduction-based stochastic surrogate model

• Main idea: Perform dimensionality reduction in the input-output space



• "Extract" stochastic surrogate model from results of dimensionality reduction

Procedures of the proposed stochastic surrogate model

- Dimensionality reduction in the input-output space–construct $\mathcal{H}: \mathbf{z} \equiv (\mathbf{x}, \mathbf{y}) \in \mathbb{R}^{n+m} \mapsto \boldsymbol{\psi}_{z} \in \mathbb{R}^{d}$
- Construct a conditional distribution $f_{\hat{Y}|\Psi_z}(\hat{y}|\psi_z)$ to predict y given ψ_z
- "Extract" a surrogate model $f_{\hat{Y}|X}(\hat{y}|x)$ from \mathcal{H} and $f_{\hat{Y}|\Psi_z}$



Dimensionality reduction



Condi. distribution

Original model

Procedures of the proposed stochastic surrogate model

- Dimensionality reduction in the input-output space–construct $\mathcal{H}: \mathbf{z} \equiv (\mathbf{x}, \mathbf{y}) \in \mathbb{R}^{n+m} \mapsto \psi_z \in \mathbb{R}^d$
- Construct a conditional distribution $f_{\hat{Y}|\Psi_z}(\hat{y}|\psi_z)$ to predict y given ψ_z
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"Stationary" surrogate model:

$$f_{\widehat{Y}|X}^{(\infty)}(\widehat{y}|x) = \int \int f_{\widehat{Y}|\Psi_z}(\widehat{y}|\psi_z) f_{\Psi_z|XY}(\psi_z|x,y') f_{\widehat{Y}|X}^{(\infty)}(y'|x) d\psi_z dy$$

"approximate" surrogate

prediction stage

Transition kernel:

$$T(\widehat{\boldsymbol{y}}^{(t)}, \widehat{\boldsymbol{y}}^{(t+1)} | \boldsymbol{x}) = f_{\widehat{\boldsymbol{Y}} | \boldsymbol{\Psi}_{z}}(\widehat{\boldsymbol{y}}^{(t+1)} | \boldsymbol{\psi}_{z}) f_{\boldsymbol{\Psi}_{z} | \boldsymbol{X} \boldsymbol{Y}}(\boldsymbol{\psi}_{z} | \boldsymbol{x}, \widehat{\boldsymbol{y}}^{(t)})$$

 \rightarrow Outputs: Stochastic surrogate model, $\hat{y}^{(t)} \sim f_{\hat{y}|x}(\hat{y}|x)$

Summary of the proposed stochastic surrogate model

$$X = [X_1, X_2 \dots, X_n] \qquad \qquad \mathcal{M}(X) \qquad \qquad Y = [Y_1, Y_2, \dots, Y_m]$$

where $n \gg 100$

- We "extract" a surrogate model from the results of dimensionality reduction
 - Surrogate model for high-dimensional system
- **Stochastic simulator**: Output predictions are probabilistic distributions
- Multi-output predictor: We can quantify interdependencies between multiple outputs

Kim, J., Yi, S. R., & Wang, Z. (2024). Dimensionality reduction can be used as a surrogate model for high-dimensional forward uncertainty quantification. *arXiv preprint*:2402.04582.

2. Uncertainty quantification of seismic response

Sources of uncertainty



Randomly paired (structural parameter + ground motion) input

Structural model and uncertain parameters

• Auburn Ravine bridge: a PSC girder bridge featuring 6 spans and 2 piers per bent (Konaki & ADK, 2011)



Box-girder cross section



Structural uncertain parameters

 $X_s = \{\text{Damping, Material prop., Geo.}\} \in \mathbb{R}^{12}$

Modeling parameter	P_1	P_2	Distribution	
Rayleigh damping ratio, ξ	0.05	0.35	Lognormal	
Cross-sectional area of the girder, A_b (m ²)	4.50	8.50	Uniform	
Elastic modulus of the girder, E_b (GPa)	28.3	0.35	Lognormal	
Elastic modulus of pier reinforcing steel, E_s (GPa)	200	0.35	Lognormal	
Yield strength of pier reinforcing steel, f_s^y (MPa)	475	0.35	Lognormal	
Ultimate strength of pier reinforcing steel, f_s^u (MPa)	655	0.35	Lognormal	
Onset of strain hardening of pier reinforcing steel, ε_s	0.0115	0.30	Lognormal	
Elastic modulus of pier concrete, E_c (GPa)	27.6	0.30	Lognormal	
Compressive strength of pier concrete, f_c (MPa)	34.5	0.30	Lognormal	
Strain at compressive strength of pier concrete, ε_0	0.002	0.25	Lognormal	
Diameter of pier column, D_{col} (m)	0.90	1.90	Uniform	
Thickness of concrete cover, c_v (m)	0.02	0.08	Uniform	

Bridge EDPs

 $\begin{aligned} Y &= [\text{Pier drift ratios}] \in \mathbb{R}^{10} \\ Y &= [\text{Girder bending moments}] \in \mathbb{R}^{25} \end{aligned}$

Ground motions

(Baker & Lee, 2018)

- **2,000 ground motions** are selected from PEER NGA-West2 database
- The target spectrum is derived from a GMM (Boore et al., 2014) and spectral correlation model (Baker & Jayaram, 2008)



Seismic hazard parameters

Parameter	Value
М	6.5
R_{rup}	10 (km)
<i>V</i> _{s,30}	450 (m/s)
Fault	Normal
Region	California

Input: structural parameter + ground motion

$$X = [X_{GM}, X_S] \in \mathbb{R}^{\geq 1,000}$$

Output: bridge EDP

 $Y = [\text{Pier drift ratios}] \in \mathbb{R}^{10}$ $Y = [\text{Girder bending moments}] \in \mathbb{R}^{25}$

Baker, J.W. and Lee, C., 2018. An improved algorithm for selecting ground motions to match a conditional spectrum. *Journal of Earthquake Engineering*, 22(4), pp.708-723.

Baker, J. W., & Jayaram, N. (2008). Correlation of spectral acceleration values from NGA ground motion models. *Earthquake Spectra*, 24(1), 299-317.

Boore, D. M., Stewart, J. P., Seyhan, E., & Atkinson, G. M. (2014). NGA-West2 equations for predicting PGA, PGV, and 5% damped PSA for shallow crustal earthquakes. Earthquake Spectra, 30(3), 1057-1085.

Surrogate modeling of seismic response

• PDR prediction by stochastic simulator ($N_{Train} = 600, N_{Test} = 1400$)



• Outputs: $Y = [PDR_1, ..., PDR_{10}]$



• Mean peaks of PDRs

Discrepance NLRHA with OpenSees ($\times 10^{-3}$		Stochastic simulator ($\times 10^{-3}$)		Error (%)		
Fiel location	Left	Right	Left	Right	Left	Right
Bent 1	3.96	3.76	3.87	3.68	2.23	2.20
Bent 2	3.90	3.76	3.81	3.67	2.22	2.21
Bent 3	3.63	3.57	3.55	3.50	2.22	2.21
Bent 4	3.34	3.37	3.27	3.30	2.20	2.22
Bent 5	3.42	3.53	3.35	3.46	2.19	2.23



800

850

900

UQ for seismic response

• Girder moment prediction by stochastic simulator ($N_{Train} = 600, N_{Test} = 1400$)



• Inputs: $\mathbf{X} = [\mathbf{X}_{GM}, \mathbf{X}_{S}]$

• Outputs: $Y = [GM_1, ..., GM_{25}]$

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Application to synthetic motions

Girder #4

Girder #7

Girder #10

Girder #13Girder #16

Girder #19Girder #22

1.5

 $\times 10^4$

 $imes 10^{-3}$

0.8

0.6

0.4

PDF

(a)

0.5

Moment M_y , kN·m

2,000 spectrum-compatible synthetic motions are generated by the ground motion generation algorithm (Yanni et al., 2024)

PDF

0

 $\times 10^{-4}$

Seismic UQ for girder moments under synthetic motions:



Longitudinal position of deck

The proposed method is effective to both synthetic and recorded motions

0.5

(b)

Reference (MCS)

1.5

Moment M_z , kN·m

Prediction

Moment M_y , kN·m

Moment M_z , kN·m

0.5

2.5

 $\times 10^4$

3. Sensitivity analysis of seismic response

Global sensitivity analysis of seismic response



Variance-based sensitivity analysis

• Sensitivity indices for each EDP with respect to each group of input uncertainties



- Inevitably *high-dimensional* integral (due to X_{GM})
- High computational complexity (Complexity = $d \times N^2$)



Stochastic simulator combined with MC sampling

Sensitivity indices of seismic response

- Inputs: $\mathbf{X} = [X_{GM}, X_S]$
- Outputs: $Y = [GM_1, ..., GM_{25}]$
- Sensitivity indices of girder responses w.r.t ground motion and structural parameter uncertainties
- Grouped indices (S_{GM}, S_S) and their interaction effect $(S_{S,GM} = 1 S_{GM} S_S)$



Application to synthetic motions

- Moment M_x Force F_r $S_{\mathbf{S}} \longrightarrow S_{\mathbf{GM}} \longrightarrow S_{\mathbf{S},\mathbf{GM}}$ Sensitivity index Sensitivity index 0.750.750.50.5**Relative contributions** 0.250.25(trends) are same Abutment Bent1 Bent2 Bent3 Bent4 Bent5 Abutment Abutment Bent1 Bent2 Bent3 Bent4 Bent5 Abutment with recorded motion Longitudinal position of deck Longitudinal position of deck case Force F_u Moment M_u Sensitivity index Sensitivity index 0.750.750.5 0.50.250.25Bent3 Abutment Bent2 Bent4 Abutment Bent1 Bent2 Bent3 Bent4 Bent5Abutment Bent1 Bent5 Abutment Longitudinal position of deck Longitudinal position of deck Error < 13%Force F_z Moment M_z Sensitivity index Sensitivity index 0.750.75 $S_{\rm GM} \ge 60\%$ $S_{\rm S} \le 20\%$ 0.50.250.25Bent1 Bent3 Bent4 Bent1 Bent2 Bent3 Bent5 Abutment Bent2 Bent5 Abutment Abutment Bent4 Abutment Longitudinal position of deck Longitudinal position of deck
- Applications to synthetic ground motions

Effectively quantify relative contributions of grouped (high-dimensional) input uncertainties

• Inputs: $\mathbf{X} = [X_{GM}, X_S]$

• Outputs: $Y = [GM_1, ..., GM_{25}]$

Summary

- The research goal is to develop an efficient stochastic simulator-based approach for seismic UQ analysis of structural systems
 - > Development of a surrogate model for the **stochastic simulator**
 - > **Uncertainty quantification** of seismic response using stochastic simulator
 - > Global sensitivity analysis of seismic response leveraging stochastic surrogate model

Reference

- Kim, J., Yi, S. R., & Wang, Z. (2024). Dimensionality reduction can be used as a surrogate model for high-dimensional forward uncertainty quantification. *arXiv preprint*:2402.04582.
- Kim, J., & Wang, Z. (2025). Uncertainty quantification for seismic response using dimensionality reduction-based stochastic simulator. *Earthquake Engineering & Structural Dynamics*, 54(2), 471-490.
- Kim, J., Su, M., Wang, Z., & Broccardo, M. (2025). Recorded versus synthetic ground motions: A comparative analysis of structural seismic responses. *arXiv preprint*:2502.19549.

Future Directions: Enhancing Seismic UQ with Neural Operators

- The proposed simulator predicts structural responses at vector scales, such as the EDP vector
 - Prediction of complete continuous structural response function remains limited
- Capturing the complete structural response function enables improved long-term maintenance planning, damage detection, and localization



Neural operator for describing continuous, infinite-dimensional response functions

Future Directions: Enhancing Seismic UQ with Neural Operators

• **Neural operators** aim to learn the underlying mathematical operator that governs the system rather than pointwise approximations: **Function-based learning**



Iterative kernel integrations \mathcal{K}_t : Encodes the mapping between features across layers

Future Directions: Enhancing Seismic UQ with Neural Operators

• Integrating **neural operators** into the stochastic simulator for predicting time-series displacement and velocity profiles, and full-field stress distribution under seismic excitation



Joint PDF for representing complex spatial and temporal patterns in structural response

Thanks for listening

