# Effect of Fines and Plasticity on Evaluating Sample Disturbance



# PEER Internship Program – Summer 2013

Undergraduate Intern: Shelly Dean, Humboldt State University Graduate Mentor: Chris Krage, UC Davis Faculty Mentor: Professor Jason DeJong, UC Davis University of California Davis



A Undisturbed (KC/NS) Undisturbed (KC/SS) 🔿 Disturbed (KC/NS)

Disturbed (KC/SS)

#### Introduction

Ground failure due to earthquake loading has occurred over a broad range of low plasticity clays and silts, indicating the need for direct data that identifies the properties and strengths of intermediate soils (see Figure 1). However, practical, scientific characterization of sample disturbance criteria for intermediate soils still needs to be developed.





Figure 1: Atterberg limits chart of soils prone to sandlike liquefaction, clay-like cyclic softening, or intermediate behavior [Boulanger and Idriss 2006].

The objective of this study is to establish a relationship between fines, plasticity, and level of sample disturbance for intermediate soils.

#### **Methods**

This study performs controlled rate of strain (CRS) consolidation tests (see Figure 2) on samples of silica silt or nevada sand with varying amounts of kaolin clay. Each mixture experiences different levels of induced sample disturbance.

Figure 5: Increasing the level of plasticity increases strain,  $C_s$ ,  $C_r$ , and  $C_c$ .



Figure 6: Only  $\Delta e/e_0$  and  $C_s$  are subject to sample

disturbance.

Figure 6: The  $C_s$  is susceptible to sample disturbance, while the C<sub>r</sub> does not change with disturbance. The  $C_s/C_r$  ratio is more dependent on level of sample disturbance than level of plasticity.

### Conclusions

 Increasing the amount of fines and level of plasticity should increase  $\Delta e/e_0$ ,  $C_s$ ,  $C_r$ , and  $C_c$ 

•Sample disturbance increases  $\Delta e/e_0$  and  $C_s$ , but does not affect C<sub>r</sub> and C<sub>c</sub>

•The  $\Delta e/e_0$  index decreases as a function of function of plasticity and should not be used to classify soils with PI values less than 15%

•From the samples tested, the  $C_s/C_r$  ratio appears to provide a better sample disturbance classification for intermediate soils than the  $\Delta e/$  $e_0$  ratio

•Although the  $C_s/C_r$  ratio seems to be a viable alternative for classifying sample disturbance for intermediate soils, more research must be performed to confirm



Figure 2: Soil sample loaded *in the GEOTAC automated* consolidation system.



Figure 3: The evaluated consolidation curve parameters are:  $\Delta e/e_0$ , swell index ( $C_s$ ), recompression index (C<sub>r</sub>), and compression index  $(C_c)$ .



Figure 7: The  $\Delta e/e_0$  index is a sample disturbance criteria developed for pure clays. Applying the  $\Delta e/e_0$ index to intermediate soils shows that the  $\Delta e/e_0$  ratio is not only dependent on level of sample disturbance, but also level of plasticity.

## Acknowledgements

This research is based upon the support of PEER and funding from the National Science Foundation. I would like to acknowledge graduate mentor Chris Krage and Professor Jason DeJong for their invaluable guidance throughout this study. Much appreciation also goes out to Heidi Tremayne for her role in establishing the support for this study.

#### References

Boulanger, R. W. and Idriss, I. M. (2006). "Liquefaction Susceptibility Criteria for Silts and Clays." J. Geotech. Eng., 132(11), 1413-1426.



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The PEER Internship Program is funded from a REU Site Award from the National Science Foundation.

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